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DEPARTMENT OF THE NAVY ATLANTIC DIVISION NAVAL FACILITIES ENGINEERING COMMAND NORFOLK, VIRGINIA

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CATHODIC PROTECTION SURVEY

at the

MARINE CORPS BASE CAMP LEJEUNE, NORTH CAROLINA

FINAL SUBMITTAL January 14, 1985

prepared by

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SUMMARY

MENENDEZ-DONNELL & ASSOCIATES, INC., in association with its consultant, GENERAL CATHODIC PROTECTION SERVICES, INC., conducted a corrosion control survey of underground POL systems, water distribution system, elevated water tanks, and underground fuel tanks at the U.S. Marine Corps Base, Camp Lejeune, North Carolina, during October and November, 1984.

The corrosion survey included inspection and evaluation of existing elevated water tanks' cathodic protection systems; inspection and testing of underground steel structures, and recommendations for cathodic protection systems for proposed new construction.

None of the POL and fuel storage facilities has cathodic protection.

The underground water distribution system has no cathodic protection, and would be the most difficult and expensive of all base piping systems to protect since it consists primarily of bare or poorly coated cast iron pipe and is not electrically continuous.

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The fourteen elevated water tanks were found to be under complete cathodic protection and with the internal coating in very good condition.

The soil resistivity tests showed a wide variation ranging from a low of 1,400 ohm-cm at Bldg. M622 in the Montford Camp area, up to 1,150,000 ohm-cm, on Snead's Road between Marine Road and Amphibian Road. Low resistivity corrosive soils below 5,000 ohm-cm constitute about 8% and moderately corrosive soils between 5,000 and 10,000 ohm-cm constitute about 21% of the totals. Laboratory tests of soil samples showed the pH to be essentially neutral, and both chloride and sulfate contents are moderate.

A new impressed current cathodic protection system should be provided for the fifteen underground steel, tanks and existing steel piping at the Fuel Farm.

New impressed current cathodic protection systems should be provided for the underground fuel storage tanks located at the Main Exchange gas station; at Bldg. No. 1885; at Bldg. No. 1775; at the Courthouse Bay area gasoline station and diesel fuel storage area; and at Bldg. FC-202, French Creek area.

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New sacrificial cathodic protection systems should be provided for the underground fuel Storage Tanks located at the Rifle Range area, at the Beach area, and at the New Naval Hospital.

Cathodic protection of the underground water piping system with sacrificial type, galvanic anodes is recommended for piping in soils with resistivities of 5000 ohm-cm or less.

Cost estimates for the recommended work are:

- 1. Install a new rectifier and groundbed on tanks and piping at the Fuel Farm; \$30,710.00
- 2. Install 5 new rectifiers and groundbeds on various fuel tanks throughout the base as previously referenced; \$36,667.00
- 3. Install a new rectifier and groundbed on tanks at the Main Exchange: \$9,640.00
- 4. Install magnesium anodes on underground Fuel Storage Tanks at the Rifle Range, New Naval Hospital and the Beach area:

\$6,553. + \$ 20,610. =

\$27,163.00



This report contains all data acquired and conclusions reached as a result of the corrosion survey of underground POL system, utility systems, water distribution systems, elevated water tanks and underground fuel storage tanks at the Marine Corps Base, Camp Lejeune, North Carolina.

Field work was started on November 5, 1984, and was completed by November 14, 1984. It consisted of collecting data and studying all existing cathodic protection systems, obtaining soil resistivity measurements, obtaining soil and water samples at selective locations, conducting continuity tests, obtaining structure-to-electrolyte potential measurements, and performing current requirement tests on line sections and selected underground storage tanks.

There are fourteen existing impressed current cathodic protection systems on the elevated water tanks. No cathodic protection exists for the following facilities:

- 1. The underground water distribution system.
- 2. Tanks and piping at the Fuel Farm.
- Various underground fuel storage tanks throughout the Base.



All data obtained during this survey is included in the Tables of Appendix B. Results and analysis of the data are included in Sections 2.1.3 and 2.2.3. The test procedures used during this survey are described in Section 2.1.2 and 2.2.2 of this report. The layouts of recommended cathodic protection systems and test points used during this survey are shown on Drawings enclosed in Appendix H of this report.

Photographs were taken of underground piping systems, elevated water storage tanks, rectifiers and various miscellaneous structures. These may be found in Appendix G.

The purpose of this survey was to evaluate the effectiveness of the existing cathodic protection systems; to determine any additional corrosion control requirements and to establish the most feasible type of additional cathodic protection systems, when required. In addition, supportive information, such as drawings, photographs, cost estimates and appropriate recommendations are supplied.



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SURVEY



CORROSION CONTROL SURVEY

2.1 POL System

2.0

2.1.1 System Description

The POL System consists of tank car unloading facilities located north of the Fuel Farm in the Industrial Area, a truck loading station, storage tanks, refueling facilities and the connecting underground piping.

MOGAS fuel is received at the Fuel Farm and stored in ten underground steel tanks of varying capacities. The total storage capacity of MOGAS Fuel is 141,000 gallons.

Diesel fuel is received at the Fuel Farm and stored in two 12,000 gallon and in two 15,000 gallon underground steel tanks.

Number 6 fuel is received at the Fuel Farm and stored in a 600,000 gallon aboveground steel tank.

Two other aboveground steel tanks No. S-1701 and S-1735, store 420,000 gallons and 172,000 gallons of No. 6 fuel respectively. In addition to the Fuel Farm storage facilities, MOGAS, Diesel, Kerosene, number 2 and number 6



fuels are stored for local use throughout the Base in tanks with capacities ranging from 2,000 gallons to 30,000 gallons. For detailed breakdown of these fuel storage facilities at each area, please refer to Inventory, Appendix A.

2.1.2 Test Procedures

Test procedures on the POL Systems included taking soil resistivity and structure-to-electrolyte potential measurements, conducting current requirement tests to determine design criteria for unprotected structures, and collecting soil and water samples for laboratory analysis.

2.1.2.1 Soil Resistivity Survey

Soil resistivity measurements were acquired at approximately 1000 ft. intervals along underground piping systems throughout the camp to 5-foot average depths, using a Nilsson Model 400 soil resistivity meter and the "Wenner" four pin method. Measurements were also acquired to 10 ft., 15 ft., and 20 ft. depths near and around all underground tanks within the POL system. The location of individual resistivity measurements are shown in Drawings No. 5000 through 5020, of Appendix H, and the soil resistivity data are presented in Table I, Appendix B.



2.1.2.2 Structure-to-Electrolyte Potential Survey

Structure-to-electrolyte potential measurements were taken on the POL system facilities, using a high impedance digital Beckman Model 3010 volt-ohm meter with reference to a saturated copper-copper sulfate half cell.

Potential measurements were taken at representative locations including piping at pumphouses, and around storage tanks. For each measurement the reference electrode was placed directly over or as near as possible to the structure subject to test. All acquired potential measurement data are presented in Table III, Appendix B. Test point locations are shown in Drawings No. 5019 & 5020

2.1.2.3 Current Requirement Tests

Current requirement tests were conducted on various underground tanks to aid in determining the Cathodic Protection design criteria for POL structures. This procedure consisted of applying direct current to the structure under test using a 12-volt automobile battery as a temporary power source and 5/8-inch diameter by 5 ft. long steel rods driven into the ground for anodes. Whenever it was necessary, abandoned lines and metal post



fences were used as temporary groundbeds to satisfy the high current demand.

Structure-to-electrolyte potential measurements were taken both before and during the application of the test current. The current output was determined by measuring the voltage drop across a calibrated 100mV-100A shunt. The current requirement was determined by the magnitude of potential shift between the native potential and the measured potential with current applied.

Generally accepted criteria for cathodic protection (NACE and DOT) used for this project, is a structure to electrolyte potential of minus 0.85 volts referred to a copper-copper sulphate half cell at all test points on the structure under test, or to achieve a minimum 300 millivolt negative potential shift with protective current applied. Current requirements test data are shown in Tables III, Appendix B.

2.1.2.4 Soil and Water Analysis

Soil samples were gathered from nine distributed locations along the POL and water distribution systems. These samples were taken at depths from 18-inches to approximately 3 ft.


Water samples were gathered from six representative elevated water tanks around the base.

The soil samples were sealed in sterile Zip Lock plastic bags and the water samples were stored in sterile glass jars. They were submitted to SGS Control Services, Inc., Houston, Texas, for chemical analysis. Specific tests were made for:

- 1. Electrical conductance
- 2. pH
- 3. Chlorides
- 4. Sulfates
- 5. Sodium
- 6. Phosphate
- 7. Carbonate

The locations from which the samples were acquired are shown on drawing No. 5000 and the chemical analysis data are presented in Appendix C.



2.1.3 Results and Analysis

2.1.3.1 Soil Resistivity Measurements

Soil resistivity is the reciprocal of soil conductance, and is usually expressed in ohm-cm. It is the most commonly used criterion for estimating the corrosivity of a given soil. The resistivity of a given soil is one of the primary factors affecting the flow of electrical currents associated with corrosion. A scale often used by corrosion engineers to classify the corrosivity of soil is as follows:

Soil Resistivity	Classification
Below 1000 ohm-cm	Extremely corrosive
1000 to 5000 ohm-cm	Very corrosive
5000 to 10,000 ohm-cm	Mildly corrosive
Above 10 000 obm-cm	Progressively less corresive

As shown on the data sheets in Table I, Appendix B, soil resistivity measurements at or near the POL facilities range from a low of 2,600 ohm-cm near the New Navy Hospital, up to 66,000 ohm-cm at the French Creek Area. With the exception of the New Navy Hospital Area, all soils measured were 10,000 ohm-cm or higher.



Serious corrosion can occur in higher resistivity soils where large variations in soil resistivity exist. These diverse resistivities indicate the existance of varying soil compositions, and such variations are conducive to concentration cell corrosion activity on the underground pipeline as it extends through the boundaries of the dissimilar soils. Corrosion is often encountered at such boundaries in the lower resistivity soils.

2.1.3.2 Structure to Electrolyte Potential Measurements

The level of cathodic protection of a given structure is evaluated by structure-to-electrolyte potential measurements. The most generally accepted criterion for cathodic protection of steel and cast iron structures buried or submerged in an electrolyte is a structure to electrolyte potential measurement of at least 0.85 volt negative to a saturated copper-copper sulfate half-cell, with DC current applied. Another widely accepted criterion for cathodic protection is a negative potential shift of 300 mv with protective current applied to the structure.

These are also two of the criteria established by NACE in its Recommended Practice R.P 01-69 (1983 REV); and also two of the criteria specified by the U.S. Department of



Transportation Office of Pipeline Safety Regulations for natural gas and hazardous liquid pipelines.

Native state structure to soil potentials are also useful in evaluating the level of corrosion occuring on an underground steel structures and therefore helpful in determining if that structure should be cathodically protected. In a given homogeneous electrolyte, anodic and cathodic areas would not develop on a steel structures if potential differences did not exist. Since the soil is not a homogeneous electrolyte, anodes and cathodes do develop with the areas with more negative potentials being the anode. The severity of corrosion is directly proportional to the difference in potential of the anodic and cathodic ares of an electrically continuous steel structures.

An analysis of the native state structure to soil potential data acquired on the POL system and presented in Table III, Appendix B, shows a wide variation in potential differences between anodes and cathodes on individual structures or systems. These range from -0.062 volts at the 10,000 bbl. tank in the Beach Area, up to -0.216 volts at the three 6,000 bbl. tanks in the Court House Bay Area. Greater potential differences probably would have been found had more potential measurements been taken.



These potential differences are large enough that moderately severe to severe corrosion can occur on the underground POL systems even in many of the higher resistivity soils unless they are cathodically protected.

A summary of known structures that should be cathodically protected is as follows:

- Underground steel tanks and associated piping in the Fuel Farm.
- 2. Four fuel tanks at Main Exchange Gas station.
- 3. Four fuel tanks at Bldg. 1855, Industrial area.
- 4. Two fuel tanks at Bldg. 1755, Industrial area.
- 5. One fuel tanks at Rifle Range gas station.
- 6. Three fuel tanks at Court House Bay gas station.
- One additional 30,000 gallon diesel fuel tank in the Court House Bay Area.
- 8. One No.2 fuel tank near Steam Plant in Beach Area.
- One No. 2 fuel tank at Bldg. FC-202 in the French Creek Area.
- 10. Six fuel tanks in the New Navy Hospital Area.
- Other miscellaneous tanks not specifically included above.



2.1.3.3 Current Requirement Tests

Current requirement test data are presented in Tables III, Appendix B. A total of six current requirements test were conducted on various underground fuel storage tanks located throughout the Base. Due to the high current demand and the high soil resistivity at the Fuel Farm Area, attempts to set up a temporary groundbed and power source were not successful. As a result, current requirements at the Fuel Farm were calculated based on .00148 ampere per square foot current density as determined by actual test previously made for Cherry Point Air Station's Fuel Farm, since the two installations are similar.

Impressed current testing of the gas station fuel tank located in the Rifle Range area and of the fuel tank located in the New Navy Hospital indicated that current drains of 0.25 amperes and 0.235 amperes, or current densities of 0.000326 amperes and 0.00033 amperes per square foot, respectively, were required to provide cathodic protection. Two other impressed current tests were conducted. One, on the three fuel tanks at the gas station located in the Courthouse Bay area, which required a current drain of 0.40 amperes, or a current density of 0.00026 amperes per square foot for cathodic protection. The other, on the four fuel tanks located at the Main



Exchange gas station in which 0.4 amperes and 0.6 amperes of current were impressed on the tanks. The data were extrapolated and 0.9 amperes of current was estimated for cathodic protection of the tanks.

Impressed current testing of fuel tank FC-202 located in the French Creek Area indicated that 0.1 ampere was not enough to achieve protective potentials. Due to the high soil resistivity (66,000 ohm-cm) the current drain obtained from a temporary groundbed was limited to 0.1 amperes. Therefore, in figuring the current requirement, current densitites calculated for other areas were considered.

Impressed current testing of the fuel tank located near the steam plant in the Beach Area indicated that the tank is shorted through the piping to the steam plant. The current requirement was therefore based on current density calculated for other areas with allowances made for the very low (1000 ohm-cm) soil resistivity measured in this area.

Calculations of tank surface areas and current densities can be found in Appendix D of this report. These calculations are based on tank dimensions and sizes provided us by base personnel.



These current density values should be used for design calculations to estimate current requirements for other underground steel tanks of similar type and environment.

2.1.3.4 Soil and Water Analysis

The nine soil samples analysis appear to be normal for this area. The soil conductivity varies from a high of 371 micro mhos/cm for sample S-18 to a low of 47 micro mhos/cm for sample S-11. Sample S-11 was obtained from the north side of the Fuel Farm. Sample S-12 was obtained from the soil backfill on top of the Fuel Farm; which is indicative by the side variation in their conductivities.

The pH values of the soil samples range from a low of 6.1, which is slightly acidic, to a high of 9.5 for Sample S-18. A pH of 9.5 is moderately basic or alkaline, but presents no problems for steel pipe or tanks.

For water sample analysis, refer to Section 2.2.3.5.

2.2 Water Distribution System

2.2.1 System Description

The water distribution system consists of facilities for



the treatment and filtration of raw water for domestic and industrial use and fire protection; and underground distribution piping. Water wells scattered throughout the base constitute the primary source of raw water.

Raw water is piped to the water reservoirs located at the filtration plants. The water is treated and filtered before being discharged to fourteen elevated water tanks. The water is then piped from the individual storage facilities to basewide facilities.

2.2.2 Test Procedures

Test procedures on the water distribution system included soil resistivity measurements, pipe-to-soil potential measurements, electrical continuity tests, internal investigation of elevated water tanks, rectifier and anode inspection, and electrolyte chemical analysis.

2.2.2.1 Soil Resistivity Survey

Soil resistivity measurements were obtained at approximately 1000 foot intervals along the right-of-way to 5 foot average depths. A Nilsson Model 400 soil resistivity meter and the Wenner four-pin method were utilized to obtain the measurements.



This procedure involved driving four steel pins into the earth in a straight line, equally spaced with the pin spacing equal to the depth to which the average soil resistivity was desired. The average soil resistivity measurement is a function of the voltage drop between the center pair of pins with current flowing between the two outside pins.

Soil resistivity measurements obtained in the vicinity of the water distribution system are listed in Table I, of Appendix B.

All test locations are shown on drawings No. 5000 to 5019, Appendix H.

2.2.2.2 Structure-to-Soil Potential Survey

Structure-to-soil potential measurements were obtained on the fire hydrants at representative locations throughout the station including the residential areas.

All potential measurements were obtained using a high input impedance voltmeter Beckman Model 3010 in conjunction with a copper-copper sulfate reference electrode placed directly over or as near as possible to the structure subject to test.



Potential measurements obtained on the water distribution system are listed in Table II of Appendix B.

All test point locations and their respective reference numbers are shown on Drawings No. 5001 to 5019, in Appendix H of this report.

2.2.2.3 Continuity Tests

Continuity tests were conducted at various locations throughout the Base. A temporary groundbed consisting of four 5 ft. long ground rods and an automobile battery were utilized. The test was performed by measuring pipe-to-soil potentials at one test point, then moving the negative connection to the next test point location with the reference electrode kept stationary. Electrical continuity between test points is indicated when both potential measurements are of the same magnitude. Electrical discontinuity between test points is indicated when potential measurements are of different magnitude. Continuity test results are shown in Table IV, Appendix B, and on Drawings No. 5001 thru 5019.

2.2.2.4 Elevated Water Storage Tank Inspection

Visual inspection of anode array, handhole inspection



plates, conduits, wiring, rectifier unit and coating integrity was performed at fourteen elevated water tanks. All observations were recorded in the field. Please refer to section 2.2.3 for Results and Analysis of this report.

2.2.2.5 Elevated Water Storage Tanks Potential Profile Survey

A potential profile of the submerged portion of each tank was conducted utilizing a standard copper-copper sulfate reference electrode in conjunction with a high impedance Beckman voltmeter (Model 3010). The reference electrode was lowered to the bottom of each tank, and tank to water potentials were measured and recorded at 3 ft. intervals to the top, along the tank wall. Data acquired are presented in Table V, Appendix B of this report.

2.2.2.6 Tank Rectifiers and Anode Strings Investigations

Each rectifier was visually inspected and adjusted to provide optimum output in accordance with potential measurements taken inside the tank.

All rectifier meters were checked and calibrated as needed, using accurate portable test meters. All meters were left



operating properly with no further repairs needed.

Voltage measurements were taken directly off the DC stacks. Direct current outputs were determined by connecting the Beckman Voltmeter across the calibrated shunts. The meters were then adjusted to reflect the findings as accurately as possible.

Individual anode strings were inspected at each tank. Anode string current drains were measured and recorded using an SWAIN Model CP-3/4 inductive clip meter. This data is presented in Table V, Appendix B.

2.2.2.7 Water and Soil Analysis

Water samples were taken from six elevated water tanks at Camp Lejuene. These samples were placed in sterile glass jars and submitted to SGS Control Services, Inc., Houston, Texas for analysis. Results are discussed in Section 2.2.3.5. Procedures for soil analysis are discussed in Section 2.1.3.4. Results of the analysis are presented in Appendix C.



2.2.3 Results and Analysis

2.2.3.1 Soil Resistivity Measurements

Soil resistivity is the reciprocal of soil conductance, and is usually expressed in ohm-cm. It is the most commonly used criterion for estimating the corrosivity of a given soil. The resistivity of a given soil is one of the primary factors affecting the flow of electrical currents associated with corrosion. Since the corrosion rate or severity is dependent on the relatioship of the potential difference between anode and cathode and the corrosion cell circuit resistance as expressed by Ohm's Law, I=E/R, and considering that soil resistivity accounts for essentially all circuit resistance; it can be stated that the corrosion rate is inversely proportional to the soil resistivity. For example, if other conditions are equal, the corrosion rate will be three times as great in 1000 ohm-cm soil as in 3000 ohm-cm soil. A scale often used by corrosion engineers to classify the corrosivity of soil is as follows:



Soil Resistivity Below 1000 ohm-cm 1000 to 5000 ohm-cm 5000 to 10,000 ohm-cm Above 10,000 ohm-cm Classification

Extremely corrosive Very corrosive Mildly corrosive Progressively less corrosive

As shown on the data sheets in Table I, Appendix B, soil resistivity measurements are generally above 10,000 ohm-cm, with only 8% below 5,000 ohm-cm and 21% between 5,000 and 10,000 ohm-cm.

Serious corrosion can occur in higher resistivity soils where large variations in soil resistivity exist. These diverse resistivities indicate the existance of varying soil compositions, and such variations are conducive to concentration cell corrosion activity on the underground pipeline as it extends through the boundaries of the dissimilar soils. Corrosion is often encountered at such boundaries in the lower resistivity soils.

2.2.3.2 Structure to Soil Potential Measurements

The discussion of cathodic protection criteria presented in Section 2.1.3.2 is also applicable to the water distribution system.



Water line potential measurements obtained throughout the Camp were, with one exception, well below the negative 0.85 volt criterion, showing a lack of cathodic protection. The exception is a single potential measurement of -0.85 volt on a water spigot at the campsite in the Beach Area, Reference No. 311, Drawing No. 5017. This measurement 1s higher than the oxidation potential of steel and is indicative of galvanized piping, or may simply be an invalid reading and should be disregarded.

Structure to soil potentials taken along a bare underground pipeline undergoing active corrosion can range from a low of -0.1 to -0.3 volts in the most cathodic areas to a high approaching -0.8 volts in the most anodic areas.

Generally speaking, older pipelines that have developed a uniform rust film will have lower average potentials than newer lines that have not developed as much rust film and consequently have more bare steel in contact with the electrolyte. Potentials measured along the water system ranged from a low of -0.200 volts to a high of -0.687 volts indicating the probability of corrosion activity in some areas.



2.2.3.3 Continuity Tests

The data acquired from continuity tests at eighteen locations (Table IV, Appendix B) show a lack of electrical continuity between joints on these sections of the water distribution system. This is typical of mechanically coupled piping, and each joint must be electrically bonded before the system can be cathodically protected with an impressed current system. Sacrificial anodes could be installed on each joint without bonding.

2.2.3.4 Elevated Water Tanks

Normally a standard inspection of a cathodic protection system installed in a water tank encompasses an electrical potential profile on three foot intervals, a visual inspection of the anodes and associated hardware, and a calibration of the rectifier to provide optimum levels of protection to the interior submerged portions of the tank. In some cases where provisions have been made by providing access covers at designated cardinal points, additional electrical potential profiles are taken to correlate readings in order to assure proper current distribution.



Visual inspection of the coating is usually noted as an aid in the overall analysis of the performance of the corrosion mitigation measures. Assuming anode array integrity, the quality of the coating will be the single greatest factor determining current distribution to the tank surfaces.

Analysis of current drain data from individual anode strings is helpful in verifying a functional anode array and, to some extent, coating integrity. Since the anodes are wired in a series-parallel configuration with the same number and size of anodes in each string of a specific "ring"; current drains should be essentially uniform if all anodes are intact and coating quality is uniform.

The findings of this report as they relate to the total current requirement to obtain effective protective levels of cathodic protection correlate coating integrity better than any other measurement used. Since in almost all cases we found that very little current was required to achieve adequate protective levels on the tank interiors, one can be reasonably assured that very little metal is exposed and the coatings are in fairly good condition.


It should be noted that the rectifier ouput data listed in the Tables under "Rectifier Data" were measured with rectifier panel meters which had been calibrated with accurate portable test meters as closely as possible, and the current drain data listed under "Anode String Current Drains" were taken with the SWAIN clamp-on meter. The total current drains do not always agree, in which case the rectifier meter is not accurate.

Data acquired on elevated water tanks are presented in Table V, Appendix B. Results and analysis on each tank are discussed in the following paragraphs.

Tank No. S-1000

Rectifier No. 4107 rated at 40 volts and 20 amperes was left operating on a transformer tap setting of A-3 providing 0.75 ampere of current to the bowl and 0.2 amperes to the riser at 4.0 volts. The potential profile data indicated that adequate protection is being achieved and individual anode string current drains confirmed anode array integrity. The anodes themselves appeared in good condition and can be expected to perform for approximately 6 to 8 more years. The associated hardware was in fair condition, however there were a few condulet covers missing on the balcony.



These should be replaced since water accumulating on the balcony can enter the conduit and make its way to the rectifier cabinet. The interior coating looked good. Structurally, the roof manway is detached, rusted and represents a hazard which should be repaired as soon as possible.

Tank No. S-29

Rectifier No. 4106 rated at 18 volts and 16 amperes was found to be operating on a tap setting of B-1. Potential measurements indicated over-protection and the transformer taps were changed to A-1. The potential profile indicated adequate levels of protection and individual anode string current drains confirmed anode array integrity. The coating system appeared to be good and the anodes themselves should last approximately 6 to 8 more years. The associated hardware such as conduit, wiring, and handhole covers were all in good condition. Structurally the tank appeared to be in fairly good condition.

Tank No. S-FC-314

Rectifier No. 7238 rated at 20 volts and 24 amperes was found to be operating on a transformer tap setting of B-2.



Measurements taken from the stacks and thru the SWAIN meter indicated errors in the rectifier meters which were calibrated to reflect actual voltage and current. The voltage was set from an indicated 4.5 volts to 7.0 volts. The bowl current meter was approximately correct and so was the riser meter. The roof ladder obstructed access to the manways therefore a potential profile could not be obtained. The anode string current drains confirmed anode array integrity, however, on the inner array one string was found to be missing. The coating appeared to be in good condition. The air vent on the top of the tank is completely rusted off and was lying on the top of the tank, secured only by the riser anode string. The vent was placed back in position but should be repaired as soon as possible. All obstruction lighting is missing. The condulet at the top of the tank is cracked and the cover is missing. Most likely the ladder hit and damaged it. The anodes themselves appeared to be in fairly good condition and should last at least five more years.

Tank No. S-BA-108

Rectifier No. 760043 rated at 40 volts and 10 amperes was found to be operating at a tap setting of 1C-4F providing 1.08 amperes to the bowl and 0.6 amperes to the riser at 8.0 volts.



The manway on top of the tank was rusted shut and could not be opened. Individual anode string current drains on the bowl anodes confirmed anode array integrity, however, the anodes could not be removed for inspection since they are too close to the insulator for clearance thru the 5-inch handhole access. The handhole covers are rusted badly and need to be replaced. The coating on the outside of the tank is peeling badly, particularly on the very top. The interior lighting system does not work and should be repaired so that the tank can be climbed safely.

Tank No. S-BB-25

Rectifier No. 4109 rated at 18 volts and 10 amperes was found to be operating on transformer tap setting A-1. The potential profile indicated adequate levels of protection and anode current drains confirmed anode array integrity. The anodes looked good and should last at least five more years, however, all of the bowl anodes are attached to the inlet pipe via a rope. The strings could not be freed. In addition there is a shovel lying on the bottom of the tank. The coating looked good as did all associated hardware.

Tank No. S-RR-44

Rectifier No. 80C-2835 rated at 40 volts and 20 amperes



was found operating on tap setting B-1. The potential profile indicated over-protection and the tap setting was changed to A-3. Adequate levels of protection were achieved at this setting. Readings were taken from the stack and thru the shunts to determine meter accuracy and calibrated as necessary. Anode string current drains confirmed anode array integrity, however, no reading could be taken on the riser since it was covered with wasps. All associated hardware looked good as did the coating.

Tank No. S-TC-1070

Rectifier No. 81C215 rated at 60 volts and 28 amperes was found to be operating on a tap setting of A-1 providing 0.24 amperes to the bowl and 0.13 amperes to the riser at 2.06 volts. The potential profile indicated less than adequate protection and the taps were changed to A-3 providing 4.38 amperes to the bowl and 1.72 amps to the riser at 8.02 volts. Anode string current drains confirmed anode array integrity and the coating appeared to be in good condition. There was one condulet cover missing on the balcony. The exterior of the riser needs painting. The anodes should last about 5-7 more years.



Tank No. S-TC-606

Rectifier No. 7236 rated at 40 volts and 12 amperes was found to be operating on transformer tap A-2 providing 0.455 amps to the bowl and 0.10 amps to the riser at 2.44 volts. The potential profile indicated less than adequate protection and the taps were changed to B-1 providing 3.0 amps to the bowl and 1.80 amps to the riser at 8.8 volts. All anodes looked good and should be expected to last approximately 5-7 more years. The anode current drains confirmed anode array integrity and the coating looked good.

Tank No. S-M-624

Rectifier No. 12210 rated at 18 volts and 10 amps was operating on a tap setting of A-4 providing 0.35 amps to the bowl and 0.050 amps to the riser at 3.53 volts. The potential profile indicated less than adequate protection and the taps were changed to B-3 providing 1.00 amps to the bowl and 0.6 amps to the riser at 6.72 volts. The individual anode current drains confirmed anode array integrity, however, life expectancy of the anodes should not be expected to exceed 2-3 more years. Some of the bowl wiring was under water but should be allright. The tank coating and hardware were in good condition.



Tank No. S-MP-4004

Rectifier No. 80C2834 rated at 40 volts and 16 amperes was found to be operating on transformer tap setting A-3 providing 0.58 amps to the bowl and 0.18 amps to the riser at 4.62 volts. The potential profile indicated adequate protective levels and the individual anode string current drains confirmed anode array integrity. All associated wiring as well as interior coating looked good. Anodes also looked good and should last 5-7 more years, however, rectifier does not function properly on lower tap settings, and it should be repaired.

Tank No. S-TT-40

Rectifier No. 5630 rated at 18 volts and 16 amperes was found to be operating on transformer tap setting A-3 providing 0.40 amps to the bowl and 0.06 amps to the riser at 3.0 volts. The potential profile indicated adequate protective levels and the individual anode string current drains confirmed anode array integrity. All associated wiring as well as the interior coating looked good.



Tank No. S-830

Rectifier No. 5201 rated at 36 volts and 16 amperes was found to be operating on transformer taps A-3 providing 1.0 amps to the bowl and 0.20 amps to the riser at 5.4 volts. The potential profile indicated adequate levels of protection and anode string current drains confirmed anode array integrity. The anodes looked good and should last 5-7 more years. All associated hardware as well as the interior coating looked good.

Tank No. S-2323

Rectifier No. 80C2833 rated at 40 volts and 20 amperes was found to be operating on transformer taps A-3 providing 0.45 amps to the bowl and 0.20 amps to the riser at 4.0 volts. The potential profile indicated adequate levels of protection and anode current drains confirmed anode array integrity. The anodes should last 5-7 more years and all associated hardware was in good condition. The interior coating also appeared to be in good condition.



Tank No. S-5

Rectifier No. 4103 rated at 18 volts and 10 amperes was found operating on transformer taps A-1 providing 0.6 amps to the bowl and 0.12 amps to the riser at 3.96 volts. The potential profile indicated adequate levels of protection and the anode string current drains confirmed anode array integrity. The inner anode array had only four functioning string, with the fifth string missing. All associated hardware looked good as did the interior coating. The anodes themselves appeared to be in good condition and should last 5-7 more years.

2.2.3.5 Water Samples Analysis

The analysis of the treated water samples may be found in Appendix C, with the analysis of all other samples tested.

The calculated resistivities of samples number W-12, W-13, W-14, W-15, W-16, and W-17 are 1355 ohm-cm, 5347 ohm-cm, 5882 ohm-cm, 2695 ohm-cm, 2817 ohm-cm, and 2777 ohm-cm, respectively. Sample W-12 nas a low resistivity, a moderate chloride and low sulfate content, a slightly basic (alkaline) pH of 8.6; and should be considered very corrosive.



The remaining samples have moderate resistivities, low chloride and sulfate contents and should be considered corrosive.

Based on this analysis, cathodic protection for the internal surfaces of the water storage tanks is needed to mitigate corrosion.

2.3 Evaluation of Activity Corrosion Control Program

2.3.1 Operating and Maintenance Practices

As part of the corrosion study, existing corrosion control maintenance practices were investigated. Information gathered from camp personnel indicated limited maintenance of the cathodic protection systems had been conducted.

A monthly inspection of the elevated water tank rectifiers is being performed by the Maintenance Department. It consists of a visual inspection, and reading and recording the DC output levels of each rectifier.



We believe that the present camp personnel are very capable of incorporating a successful corrosion control maintenance program with the aid of corrosion control short courses, in-field supervised training and proper cathodic protection testing equipment.



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Confidential Records Management, Inc. New Bern, NC 1-888-622-4425 9/08 RECOMMENDATIONS



RECOMMENDATIONS

3.1 POL System

3.0

Based on the results of this survey, we recommend that cathodic protection systems be installed on all underground steel tanks and POL piping. A combination of sacrificial galvanic anodes in low resistivity soils and impressed current systems in higher resistivity soils should provide the most cost effective approach.

The sacrificial anodes should be elongated, high potential magnesium anodes, prepacked in prepared backfill, such as DOW Galvomag-Galvopak, or equal.

Anodes for impressed current cathodic protection systems should be 3-inch diameter by 60 inches long specially treated graphite anodes, meeting MIL. SPEC. MIL-A-18279C. Impressed current anode backfill should be calcined fluid petroleum coke.

Specific recommendations are:

 Install a rectifier rated at 120 volts and 40 amperes output in conjunction with a distributed groundbed containing a minimum of thirty graphite



anodes and fourteen test stations for protection of the underground tanks and piping at the Fuel Farm.

- 2. Install a rectifier rated at 10 volts and 5 amperes, eight graphite and four test stations anodes to protect the four underground fuel storage tanks at the Main Exchange Gas Station.
- 3. Install a rectifier rated at 10 volts and 5 amperes with eight graphite anodes and four test stations to protect the four underground fuel storage tanks at Building No. 1855 in the Industrial area.
- 4. Install a rectifier rated at 10 volts and 5 amperes, six graphite anodes and two test stations, to protect the two underground fuel storage tanks at Building No. 1775.
- 5. Install twelve 20 lb. elongated high potential magnesium anodes, DOW Galvomag 20-D2, or equal, and one test station on the underground fuel storage tank at the Rifle Range Area Gas Station.



- 6. Install one 10 volt, 5 ampere rectifier, six graphite anodes and three test stations to protect the three underground fuel storage tanks at the Courthouse Bay Gas Station.
- 7. Install one 10 volt, 5 ampere rectifier six graphite anodes and one test station to protect the 30,000 gallon diesel fuel storage tank located in the Courthouse Bay area.
- 8. Install one 20 volt, 5 ampere rectifier, six graphite anodes and one test station to protect the underground fuel tank at Building FC-202 located in the French Creek area.
- 9. Install six 20-D2 magnesium anodes and one test station on the underground fuel tank located near the New Naval Hospital.
- 10. Install twenty 40-D3 magnesium anodes and four test stations on the five underground fuel tanks located near the New Naval Hospital.
- 11. Install nine 40-D3 magnesium anodes and one test station on the fuel tank located near the steam plant in the Beach area.



- 12. Install insulating flanges on the lines located at the above fuel tank in order to isolate it from above ground piping and the steam plant.
- 13. Install cathodic protection systems on any additional underground fuel tanks not specifically referenced above. Design criteria in Appendix D should be followed.

3.2 Water Distribution System

Recommendations for the water distribution system are as follows:

- Inspect elevated water tanks and rectifiers on a monthly basis in order to insure uninterrupted protection. Maintain current outputs as listed in Table V, Appendix B unless a change in current requirements is indicated by subsequent cathodic protection surveys.
- Replace missing or depleted anode strings in elevated water tanks as follows:
 - Tank S-FC-314: Replace one missing string in inner array.



- b. Tank S-5: Replace one missing string in inner array.
- 3. Repair or replace tank hardware as follows:
 - a. Tank S-1000: Replace 3/4-inch conduit covers on the balcony.
 - b. Tank S-FC-314: Repair the roof ladder and the air vent on top of tank, and replace the damaged condulet on top of tank.
 - c. Tank S-BA-108: Repair manway cover on top tank so it can be opened, replace the handhole covers on top of the tank, and repair the interior lighting system.
 - d. Tank S-1070: Replace one condulet cover on the balcony.
 - e. Tank S-TT-40: Replace the missing bolt and bar on the riser cover assembly.
 - f. Tank S-MP-4004: Repair existing rectifier to achieve proper operation at all tap settings.



4. Install sacrificial high potential magnesium anodes on individual underground pipe joints in all areas where soil resistivities are below 5000 ohm-cm as described in Appendix D.

As an alternate, all pipe joints falling within, and adjacent to, areas with soils below 5000 ohm-cm could be electrically bonded and cathodically protected with impressed current systems. However, both initial costs and maintenance costs will exceed the cost of sacrificial anode systems and changes of stray current corrosion will be greatly increased.

5. In areas where cathodic protection is to be considered, electrically bond all cast iron pipe joints exposed by maintenance or construction activities. Bonds should be minimum No. 8 AWG copper wire or equivalent copper straps. Electrical continuity of underground piping cathodically protected with sacrificial anodes is desirable since it equalizes structure-to-soil potentials and permits monitoring the effectiveness of the system without the need to contact each pipe joint.


 Install two-wire potential test stations at preselected locations to monitor the level of cathodic protection and anode outputs.

3.3 Activity Corrosion Control Program

3.3.1 Recommendations for Maintenance Practices

The following recommendations are aimed towards aiding Camp personnel in developing a total corrosion control preventive maintenance program.

It is recommended that the responsibility for monitoring and maintenance of cathodic protection systems, once they are installed, be assigned to competent permanent personnel with either experience in cathodic protection or with technical backgrounds to facilitate their training as described in Section 3.3.2.

The present policy of monthly rectifier inspections should be continued. These inspections should include as a minimum, reading and recording the D.C. output levels as indicated by the panel meters, and a visual inspection of all major rectifier components. Output levels should be promptly compared with those recorded from previous inspections and any significant changes investigated.



In addition, other system components should be observed and repairs effected whenever needed.

It is further recommended that a comprehensive system-wide corrosion control survey be conducted on an annual basis by an experienced corrosion engineer. The corrosion engineer accomplishing this survey should be accompanied by the station personnel responsible for corrosion control monitoring since this would constitute valuable field experience.

Drawings provided in this report showing the location of structure-to-electrolyte potential measurements should be used as a guide in the annual survey.

It is recommended that all data pertaining to the corrosion control program be recorded for future reference. The corrosion control records program should include investigating and recording all leaks that occur. Bell hole inspections should be made and a leak report form completed, detailing the type of leak, repairs made, and their locations.



For further details in establishing a corrosion control program and for additional information on maintenance programs, refer to NAVFAC INST 11014.51 of 19 October 1983 and MO-307 of May 1981; "Cathodic Protection Systems Maintenance".

Additional assistance in establishing a corrosion control program may be obtained from the Atlantic Division, Naval Facilities Engineering Command corrosion engineer.

3.3.2 Recommendations for Training Program

The routine monitoring of cathodic protection systems is essential to maintaining adequate protection against corrosion attack in soil and water electrolytes. It is recommended that a training program involving Camp personnel be instituted. This program would involve the training of personnel, in both theory of cathodic protection and field training.

The following corrosion control courses are recommended for Camp personnel.

National Association of Corrosion Engineers (NACE) Courses:



- a. "Basic Corrosion Course".
- b. "Corrosion Prevention by Cathodic Protection".
- c. "Corrosion Prevention by Coatings".

We recommend these courses for learning the basic theory of corrosion and methods and practices used in cathodic protection. These courses can be taken by "Home Study" with personnel working at their own pace. The courses are designed for people with no prior knowledge of cathodic protection. Further information can be obtained by writing to NACE Education Department, P. O. Box 218340, Houston, Texas 77218; or by telephoning (713) 492-0535.

Another excellent training course is the "Cathodic Protection Rectifier School" offered by Good-All Electric, Inc.

This short three day course is designed to familiarize students with cathodic protection rectifiers. Basic theory is discussed as well as field troubleshooting. Additional information can be obtained by writing to GOOD-ALL Electric, Inc., 3725 Canal Drive, Fort Collin, Colorado 80524, attention to Mr. Don Olson, or by calling (303) 484-3080.



A number of corrosion control short courses are offered every year by several universities and sections of NACE throughout the United States.

One of the better ones is held each May in Morgantown, West Virginia; and another excellent course is offered each September at the University of Oklahoma, Norman, Oklahoma. These three-day seminars are taught by professional instructors and include practical field demonstrations. Details of these courses can be obtaind by contacting the University of West Virginia or the University of Oklahoma, respectively.

It is also recommended that an experienced corrosion engineer accredited by NACE as a Corrosion Specialist conduct an on-site training seminar with Camp personnel. By this seminar, Camp personnel can obtain practical training on the testing procedures used for conducting routine maintenance of cathodic protection systems. This training would include taking structure-to-electrolyte potentials, soil resistivity measurements and the basics of rectifier inspection techniques.



Additional details on training courses offered by the Atlantic Division, Naval Facilities Engineering Command, the Naval Civil Engineering Laboratory, the U.S. Air Force Institute of Technology and commercial firms may be obtained by contacting the Atlantic Division, Naval Facilities Engineering Command corrosion engineer.



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ESTIMATES



4.1 Fuel Farm

4.0

- Based on the detailed Cost Estimates included in Appendix E, the initial cathodic protection investment is = \$30,710.
- 2. Investment = Initial Cost x Capital Recovery Factor thus on the basis of 12% for 20 years, the annual cost to own becomes:

 $30,710 \times 0.1175 = 33,608.$

Maximum Power Cost:

AC Watts = DC Watts conversion efficiency (.68)

Recommended Rectifier (120V-40A)

AC $KW = \frac{120 \times 40}{.68} \times \frac{1KW}{1000W} = 7.06KW$

Annual Power Cost:

 $\frac{7.06 \text{ KW x}}{\text{yr}} = \frac{8760 \text{ hr}}{\text{KW-hr}} = \frac{\$3710}{\text{KW-hr}}$

Estimated Annual Cosc=3608 + 3710 = \$ 7,318.



- Repairs and replacements on the POL system have been made in the past, but exact costs were not available.
- 4. The investment involved in the tanks and associated equipment, along with their importance to operations, justify the recommended cathodic protection system.
- DOT Standards require all underground fuel gas storage and piping to be provided with cathodic protection.

4.2 Fuel Storage Tank at Rifle Range Area

Field data indicates that two cathodic protection alternatives can be used to protect the fuel tank at the Rifle Range area.

A. Impressed Current System

 Based on the detailed Cost Estimates included in Appendix E, the initial cathodic protection investment is

= \$6,928.



2.

Investment = Initial Cost x Capital Recovery Factor thus on the basis of 12% for 20 years, the annual cost to own becomes:

\$6,928. x .1175 = \$814.

Maximum Power Cost:

AC Watts = DC Watts/conversion efficiency

Recommended Rectifier = 10V - 4A

AC KW = $\frac{10x4}{.68}$ x $\frac{1-KW}{1000}$ = 0.06 KW

Annual Power Cost:

0.06 KW x $\frac{8760 \text{ hr}}{1 \text{ yr}}$ x $\frac{\$0.06}{\text{KW-hr}}$ = 32.

Estimated Annual Cost = 814. + 32. = \$846.

B. <u>Sacrificial Anode System</u>

1.

Initial Cathodic Protection Investment as estimated in Appendix E of this report \$ 6,553.



2. Investment = Initial Cost x Capital Recovery Factor thus on the basis of 12% for 20 years, the annual cost to own becomes:

\$ 6,553 x .1175 = \$ 770.

- C- Based on the above estimated annual costs we recommend that the sacrificial anode system be installed.
- D. Annual maintenance costs of the fuel tank were not available, however if the investment involved in the tank justifies the \$770. annual cost, we recommend that a cathodic protection system be installed.
- 4.3 Fuel Storage Tanks in New Naval Hospital and Onslow Beach Areas
- 1. Based on detailed Cost Estimates included in Appendix E, the initial investment for the sacrificial cathodi protection in the two areas is = \$ 20,610.



2. Investment = Initial Cost x Capital Recovery Factor thus on the basis of 12% for 20 years, the annual cost to own becomes:

 $20,610 \times .1175 = 2,422.$

- 3. Costs of repairs and replacements on the POL system were not available. The investment in the tanks and associated equipment, along with their importance to operations, justify the recommended cathodic protection system.
- DOT Standards require all underground fuel gas storage and piping to be provided with cathodic protection.

4.4 Fuel Storage Tanks at Main Exchange

1. Based on detailed Cost Estimates included in Appendix E, the initial investment for the sacrificial cathodic protection of these tanks is = \$ 9,640.



3.

2. Investment = Initial Cost x Capital Recovery Factor thus on the basis of 12% for 20 years, the annual to cost to own becomes:

 $\$ 9,640. \times .1175 = \$ 1,133.$

Maximum Power Cost:

AC Watts=DC Watts/conversion efficiency

Recommended Rectifier = 10V - 4A

AC $KW = \frac{10x4}{.68} \times \frac{1-KW}{1000} = 0.06KW$

Annual Power Cost:

0.06 KW x $\frac{8760 \text{ hr}}{1 \text{ yr.}}$ x $\frac{\$0.06}{\text{KW-hr}}$ = \$32./yr

Estimated Annual Cost: = 1133 + 32 = \$ 1,165.

Repairs and replacements of the tanks have been made in the past, but exact cost were not available.



- 4. The investment involved to protect these tanks and associated equipment, justify the recommended cathodic protection system.
- 5. DOT Standards require all underground fuel gas storage and piping to be provided with cathodic protection

4.5 Remaining Fuel Storage Tanks

- 1. Based on the detailed Cost Estimates included in
 Appendix E, the initial cathodic protection
 investment is = \$36,667.
- 2. Investment = Initial Cost x Capital Recovery Factor thus on the basis of 12% for 20 years, the annual cost to own becomes:

 $36,667.00 \times .1175 = $4,308.$

Maximum Power Cost:

AC Watts = DC Watts/conversion efficiency.



Recommended Rectifiers = 4 each 10V-4A

1 each 20V-4A

AC $KW = \frac{4(10x4) + (20x4)}{.68} \times \frac{1}{1000} \times \frac{1}{10$

Annual Power Bill:

0.353 KW x 8760 hr/yr x \$0.06/KW-hr = \$185.

Estimated Annual Cost= \$4,308 + \$185 = \$4,493.00

3.

Leaks and repairs have been reported at several locations. Some underground fuel tanks are scheduled to be replaced with aboveground tanks or with underground fiberglass tanks. Only existing metal tanks not scheduled for replacement were considered for cathodic protection.

4. Annual replacements and maintenance costs were not available. However, if the investment involved justifies the annual cost of \$4,493. we recommend that cathodic protection systems be installed.



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APPENDIX A

INVENTORY



APPENDIX A

CAMP LEJUENE, NORTH CAROLINA

POL SYSTEM INVENTORY OF MAJOR PRODUCT STORAGE FACILITIES

				AG: Aboveground
Area	Ref. No.	Capacity	Products	UG: Underground
		(Gallons)		
Industrial	S-1009	600,000	#6 Fuel	AG-Steel
Industrial	S-1023	12,000	MOGAS	UG-Steel
Industrial	S-1024	15,000	MOGAS	UG-Steel
Industrial	S-1025	12,000	MOGAS	UG-Steel
Industrial	S-1026	15,000	MOGAS	UG-Steel
Industrial	S-1027	15,000	MOGAS	UG-Steel
Industrial	S-1028	15,000	MOGAS	UG-Steel
Industrial	S-1029	15,000	MOGAS	UG-Steel
Industrial	S-1030	12,000	MOGAS	UG-Steel
Industrial	S-1031	15,000	MOGAS	UG-Steel
Industrial	S-1032	15,000	MOGAS	UG-Steel
Industrial	S-1033	12,000	Diesel	UG-Steel
IndustriaL	S-1034	12,000	Diesel	UG-Steel
Industrial	S-1035 :	15,000	Diesel	UG-Steel
Industrial	S-1036	15,000	Diesel	UG-Steel
Industrial	S-1037	(2) 3,500	Kerosene	N/A
Industrial	Main Exchange	30,000	MOGAS	UG-Steel
Industrial	Main Exchange	30,000	MOGAS	UG-Steel
Industrial	Main Exchange	(2)10,000	MOGAS	UG-Steel
Industrial	Bldg. 1855	(2) 6,000	Diesel	UG-Steel
Industrial	Bldg. 1855	(2) 6,000	MOGAS	UG-Steel
Industrial	Bldg. 1775	6,000	Diesel	UG-Steel
Industrial	Bldg. 1775	6,000	MOGAS	UG-Steel
Industrial	S-1701	420',000	# 6 Fuel	AG-Steel
Industrial	S-1735	172,000	# 6 Fuel	AG-Steel
Old Hospital	(Not in use)	10,000	Diesel	UG-Steel
Old Hospital	(Not in use)	10,000	Diesel	UG-Steel
Berkley Manor	Exchange # 2	(3)10,000	MOGAS	UG-Steel
Berkely Manor	Exchange # 2'	10,000	Diesel	UG-Steel
Paradise Pt.	Bldg. 2615	8,000	# 6 Fuel	UG-Steel
Paradise Pt.	Bldg. 2615	8,000	# 6 Fuel	UG-Steel
Montford Pt.	M-625	30,000	# 6 Fuel	UG-Steel
Montford Pt.	M-625	20,000	# 6 Fuel	UG-Steel
Montford Pt.	M-230	15,000	Diesel	UG-Steel
Montford Pt.	M-230	15,000	Diesel	UG-Steel
Geiger Camp	Strange and service and the	(2)15,000	Diesel	AG-Steel
Geiger Camp		(2)15,000	Unlead MOGAS	AG-Steel
Geiger Camp		15,000	Kerosene	AG-Steel
Rifle Range	Gas Station	10,000	Unlead MOGAS	UG-Steel
Rifle Range	RR-15	10,000	# 6 Fuel	UG-Steel
Rifle Range	RR-15	10,000	# 6 Fuel	UG-Steel





Area	Ref. No.	Capacity	Products	UG: Underground
		(Gallons)		
Courthouse Bay	BB-9	(3)10,000	# 6 Fuel	UG-Steel
Courthouse Bay	BB-9	30,000	Diesel	UG-Steel
Courthouse Bay	BB-9	(3) 6,000	MOGAS	UG-Steel
Onslow Beach	BA-106	10,000	Diesel	UG-Steel
French Creek	FC-202	10,000	Diesel	UG-Steel
New Hospital	M7-1	(2)20,000	# 6 Fuel	UG-Steel
New Hospital	M7-1	(2)20,000	Diesel	UG-Steel
New Hospital	M7-1	10,000	MOGAS	UG-Steel
New Hospital	M7-1	2,000	Diesel	UG-Steel

WATER DISTRIBUTION INVENTORY OF STORAGE FACILITIES

Description

Capacity

Туре

	Tank	S-1000	300,000	gal.	Elevated	Steel
	Tank	S-29	300,000	gal.	Elevated	Steel
	Tank	S-FC-314	300,000	gal.	Elevated	Steel
	Tank	S-BA-108	100,000	gal.	Elevated	Steel
1	Tank	S-BB-125	100.000	gal.	Elevated	Steel
	Tank	S-BB-44	100.000	gal.	Elevated	Steel
	Tank	S-TC-1070	100.000	gal.	Elevated	Steel
	Tank	S-TC-606	100.000	gal.	Elevated	Steel
	Tank	S-M-624	150,000	gal.	Elevated	Steel
	Tank	S = TT = 40	250,000	gal.	Elevated	Steel
	Tank	S-MD-4004	200,000	gal.	Elevated	Steel
	Mank	C-930	300,000	gal	Elevated	Steel
	Tank	5-030	200 000	gal.	Elevated	Steel
	Tank	5-2323	300,000	dal	Elevated	Steel
	Tank	5-5	500,000	yur.		





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APPENDIX B

DATA SHEETS

Soil Resistivity	TABLE	I
Structure-to-Electrolyte Potential Measurements (Water)	TABLE	II
Current Requirements Tests Fuel Tanks	TABLE	111
Continuity Test, Water	TABLE	IV
Elevated Water Storage Tanks Data	TABLE	v



SOIL	RESISTIVITY MEASUREMENTS	5.4	0				
DATE	11/6/84 ENGINEER CM/J	H	TA	BLE I	PA	GE	OF 31
TEST NO.	TEST LOCATION	AVE	RAGE PTH	READING	MULTI.	FACTOR	а онм-см
1	HOLCOMB & SNEADS FERRY	5	!3"	8.40	10.0	1000	84.000
2	SNEADS & MICHAEL			2.70	:1		27000
3	LOUIS & MULBERRY ST.			1.30			13 000
4	DUNCAN ST. @ BLDG. 1012			4.35	S		43.50
5	BIRCH & LOUIS ROAD			1.25			12 500
6	ASH @ BLDG. 1114			2.10			21 000
7	ASH & HOLCOMB BUND			3.95			29 500
8	OFF HOLCOMB (BLDG. GOI)		~	8,00	1.0		8.000
9	DOGWOOD ST. @ BLDG 1400			9.30	1.0		9 300
10	DOGWOOD ST. & HAMMOND RD.			3.50	10.0		35,000
11	"O"ST. & DOGWOOD			3.50	1		35,000
12	LOUIS ROAD & GUM ST			1.20			12,000
13	GUM ST. @ BLDG. 1705			9.60	1.0	1. 20	9,600
14	GUM ST. & HALCOMB BLVD			1.20	10.0		12,000
15	MOLLY PITCHER DRIVE @ BLDG 59		-	2.00	10.0		20,000
	a an ann ann an Airtean an Airtean an Airtean an Airtean an Airtean an Airtean Airtean Airtean Airtean Airtean						
				1			1.11
-	1 -	-					
NOTES :	Nilsson 400 meter & the 4Pin metho	od w	ere	used to o	btain so	il resist	ivity



TITL	E: CATHODIC PROTECTION SURVEY,	MARINE	CORPS BAS	SE, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS	0 10	-1 2	<u>Ser Miller</u>	an Barana an Anna an A Anna an Anna an	and the second second
STR	DETURE: HAUNOI POINT	Z, AR	EAD		-	
DATE	<u>11/6/84</u> ENGINEER <u>NE/G</u>		ABLE	PA(3E <u>2</u> (OF <u>31</u>
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	ОНМ-СМ
20	HOLCOMB BLVD	5-3	4.1	1.0	1000	4,10
	~	10:6	2.8		2000	5,60
21	FIELD @ BLDG. 1725	5!3	8.3		1000	8,30
		10-6"	4.3		2000	8,60
3.5		15-9"	2.9	-	3000	8,70
	4	21:0"	2.3		4000	9,20
22	FIELD @ BLDG 751	5.3"	1.2	10.0	1.000	12,00
		10.6"	4.4	1.0	2000	8,80
		15-9"	2.3		3000	6,90
		21'-0"	1.2		4000	4,80
23	MAIN SERVICE ROAD	5-3"	1.0	10.0	1000	10,00
24			1.6			16,00
25	LOUIS ROAD		1.6	1000		16,00
26	C BLDG 1820		3.6			36,000
21	MAIN SERVICE ROAD		1.1	-		11,000
20			4.9	1.0		4,900
29	"U" SIKEEI		5.1		-	5, 100
20			5.6			5,60
21	"U SIKEET & KIVER ROAD		6.2			6,20
02	"N" STREET		9.7			9,700
55	BLDG. 540		5.7			5,700
34	"N" STREET		7.1			7,10



SOIL	RESISTIVITY MEASUREMENTS	2				
STRU	ICTURE: HADNOT POINT 2	, ARE	A 3	ing di Sang Manggang di Sang Ng		
DATE	II/0/84 ENGINEER NE/		BLE 1	PA	GE <u>3</u> 0	F <u>31</u>
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	ОНМ-СМ
35	"M" STREET	5'3"	3.3	10.0	1000	33,000
36	"L" STREET		1.3			13.000
37	"L" STREET @ BLDG 417	and the	1.2			12,000
38	"K" STREET		8.7	1.0		8,700
39		1.12	2.0	10.0		20,000
40	RIVER ROAD & "I" STREET		1.6	1		16,000
41	"I" STREET	and the second	9.2	1.0		9.200
42	"I" STREET & "H" STREET		3.6			3,600
		10-6"	3.8		2000	7.600
		15'-9"	2.6	-	3000	7 800
	\rightarrow	21-0"	2.4	~	4000	9,600
43	"H" STREET	5-3"	1.1	10.0	1000	11,000
44	river road	+	1.4	4		14,000
	-	10-0"	6.8	1.0	2000	13,600
45	HOLCOMB BLVD	5'3"	1.9	10.0	1000	19,000
16		~	9.1	1.0	\downarrow	9,100
·		10:6"	4.0		2000	8,000
		15'-9"	2.1		3000	6,300
	+	21'-0"	1.3	-	4000	5,200



TITL	E: CATHODIC PROTECTION SURVEY,	MARINE	CORPS BA	SE, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS			Ser Pro		and the second second
STR	UCTURE: HADNOT POINT	<u>r I,</u>	AREA	4		CALIFIC MARSON
DAT	E 11/7/84 ENGINEER NE/G	i <u>G</u>	TABLE I	PA	GE 4 0	DF 31
TEST NO.	TEST LOCATION	AVERAG DEPTH	E READING	MULTI.	FACTOR	онм-см
50	RIVER ROAD	5!-3	8.0	1.0	1000	8.000
51	"G" STREET		4.1			4 100
52	MAIN SERVICE RD & "G" ST.		6.5			6.500
53	"F" STREET	12.05	8.3		dan inst	8.300
54	"E" STREET		1.9	10.0		19,000
55			1.1			11,000
56	RIVER ROAD		4.0	1.0		4.000
1		10:0	3.7		2000	7.400
		15'9'	2.9		3000	8,700
	-	21-0	2.3		4000	9,200
57	"D" STREET	5!3	1.5	10.0	1000	15,000
58			1.2		,	12,000
59	POST LANE		6.1	1.0		6,100
60			3.4			3,400
61	LUCY BREWER AVE.	54	7.4	No. Sta		7,400
62	MOLLY PITCHER DR.		6.9			6,900
03	VIRGINIA DARE DR.		6.6	+	1	6,600
64	MAIN GERVICE ROAD		1.5	10.0		15,000
65	"C" STREET		6.8	1.0		6,800
106	"B' STREET		7.3			7,300
67			9.1	1.00		9,100
UB	"A" STREET	\checkmark	7.2	-		7,200
					5.55	

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TITL	E: CATHODIC PROTECTION SURVE	Y. MARINE C	ORPS BAS	E. CAMP	LEJEUNE	. N.C.
SOIL	RESISTIVITY MEASUREMENTS					
STRU	ICTURE: HADNOT POINT	TI, AR	EA 4			1
DATE	11/7/84 ENGINEER NE/	166 TI	BLE I	PA(BE <u>5</u> 0	of <u>31</u>
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	онм-см
69	"A" STREET	5!3"	3.0	1.0	1000	3,000
70	RIVER ROAD		4.7		and the second	4.700
71	NEAR WALLACE CREEK		6.5	4		6,500
72	CROSS STREET		2.1	10.0		21,00
73	MAIN SERVICE ROAD		2.5			25,00
		10:6"	1.0	+	2000	20,00
		15-9"	4.9	1.0	3000	14,70
		21'-0"	3.4	1944 - T	4000	13,60
74		5!3"	6.8		1000	6,80
		10-6"	4.9		2000	9,80
6		15! 9"	3.1		3000	9,30
		21-0"	2.2	+	4000	8,80
1						
!						
:						
1						
		Acres 1	1.00			



TITL	E: CATHODIC PROTECTION SURVEY,	MARINE C	ORPS BAS	E, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS				<u> </u>	an a
STRU	ICTURE: OLD NAVAL HOSPIT	AL A	REA 5	<u> </u>		
DATE	<u>11/7/84</u> ENGINEER <u>NE/G</u>		BLE I	PAG	DE 6 0	F 31
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	ОНМ-СМ
80	RIVER ROAD	5!3"	1.2	10.0	1000	12,000
81	RIVER ROAD @ BLDG 15		3.5	1.0		3.500
		10:6"	2.1		2000	4 200
		15'9"	1.9		3000	5700
		21-0"	1.6	1	4000	6400
82	RIVER ROAD	5'.3"	2.0	10.0	1000	20,000
83	- · ·		2.3			23,000
84	SURGEONS ROW		2.2			22,00
85	RIVER ROAD		2.4			24,000
86			3.4			34,00
87	CUTLER STREET		1.2			12,00
88	×		5.3			53,000
89	BLACKWOOD ROAD		2.0			20,000
90	\downarrow		2.6			26,000
	THE CONTRACT OF THE CONTRACT OF			19		
			:			
		13.1	i			
				1.000	1	Second



TITL	E: CATHODIC PROTECTION SURVI	EY, MARINE	CORPS B	ASE, CAM	LEJEUN	E, N.C.
SOIL	RESISTIVITY MEASUREMENTS			<u></u>	<u>laten in b</u>	<u></u>
STR	UCTURE: OFFICER'S	QUART	ERE	ABEA	6	
DATE	E <u>11/7/84</u> ENGINEER <u>NE</u>	<u>/66</u> 1	ABLE]	PA	GE 7	OF 31
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	G MULTI.	FACTO	R OHM-CM
100	SETH WILLIAMS BLVD	5!3"	6.2	1.0	1000	6,200
101	ONSLOW DRIVE		3.2	10.0		32,000
102			4.1			41,000
103			2.8		:	28,000
104	STONE STREET		8.2	1.0		8,200
105	TIMMERMAN PLACE		4.7	10.0		47,000
106	SETH WILLIAMS BLVD.		9.8	1.0		9,800
107	EDEN STREET		1.4	10.0		14,000
108	BEVIN STREET		2.1		1.2	21,000
109			3.6			36,000
110	HILL STREET		1.8			18,000
111	BEVIN STREET		5.7			57,000
112	EDEN STREET		10.2	1.0		10,200
113	SETH WILLIAMS BLVD		3.6	10.0		36,000
114	CUKELA CIRCLE	8.1	2.6			26,000
115	CUKELA STREET		3.1			31,000
116			3.4			34,000
117	SETH WILLIAMS BLVD		3.6			36,000
18			4.5	+		45,000
19	JEWEL & EDEN		2.1	100.0	\downarrow	210,000
		10-6"	5.8	10.0	2000	116,000
		15! 9"	2.5		3000	75,000
	\downarrow	21-0"	1.6		4000	64,000



TITLI SOTI	E: CATHODIC P	ROTECTION SURY	YEY, MARIN	EC	ORPS BAS	E, CAMP	LEJEUNE	, N.C.
STRU	CTURE: OF	FICERS QUI	ARTERS	3	ABEA	6		
DATE	11/7/84	ENGINEER N	E/GG	TA	BLE I	PAG	E 8 0	F <u>31</u>
TEST NO.	TEST L	OCATION	AVER DEP	AGE TH	READING	MULTI.	FACTOR	онм-см
120	EDEN ST	REET	5!	3"	5.3	10.0	1000	53.000
121					3.0			30 000
122	SETH WILL	IAMS BLVD			8.9	1.0		8.900
			10-0	, H	8.0			8,000
			15-0	7"	5.2			5,200
			21-0	2"	4.1	+		4,100
23			5-	3"	1.3	10.0		13,000
24	WINSTON	ROAD			1.6	+		16,000
							25 inter	
		· · · ·			C. A. M.			
				_				
					210			
			N					
						in the second		
	and the second sec		1			Same 2		
	North Contraction and			-+			8.0	
12/13				+				
		andre andre statistical de la constatistica de la constatistica de la constatistica de la constatistica de la c En la constatistica de la const En la constatistica de la const						and a second second Second second
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				+				
				8.1		6.00 Carl		



GC	PS GENERAL CATHODIC PROTE	ECTION S	ERVICES	S INC.		
TITL	E: CATHODIC PROTECTION SURVEY,	MARINE C	ORPS BA	SE, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS					· 20
STR	UCTURE: PARADISE POI	NT AR	EA 7		a da ang ang ang ang ang ang ang ang ang an	a taka an San Rika
DATE	11/7/84 ENGINEER NE/G	G TI	ABLE 1	PA	GE 9	OF <u>31</u>
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	онм-см 5200
130	SETH WILLIAMS @ BLDG 2702	5.3"	5.2	1.0	1000	
131	WAVEL ST. CBLDG. 2616		5.6	10.0		56.000
132	SETH WILLIAMS BLVD		3.6	1.0		3,600
133	SETH WILLIAMS & CHARLES ST.		2.9			2,900
134	SETH WILLIAMS & HOWARD ST.		4.5			4,500
35	SETH WILLIAMS & BEACH		8.2			8,200
		10-6"	2.5		2000	5,000
		15'-9"	1.3		3000	3,900
		21-0"	1.1	4	4000	4,400
36	SETH WILLIAMS BLVD	5-3"	1.2	10.0	1000	12,000
37			2.3		1.00	23,000
38	KENT ROAD		3.3			33,000
39			4.8			48,000
40	BREWSTER BLVD CGOLF CRSE		2.9			29,000
41			10.5	1.0		10,500
		10-6"	6.5		2000	13,000
		15-9"	5.1		3000	15,300
		21-0"	3.7		4000	14,800
42		5-3"	9.1		1000	9,100
43	ST. MART'S DRIVE		1.5	+		1,500
44			1.5	10.0		15,000
45			9.1	1.0		9,100
46			1.1	10.0		11,000
47			7.0	1.0		7000

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STR	UCTURE: BERKELEY MANOR	\$ WA	TKI	NS VILLA	GE ARE	EAS 84	8A			
DAT	ALL 11/1/84 ENGINEER OMULH TABLE I PAGE O OF									
NO.	TEST LOCATION	AVER/ DEP	AGE TH	READING	MULTI.	FACTOR	ОНМ-СМ			
150	BREWSTER & STONE	5-3	5"	1.8	10.0	1000	18,00			
151	ALABAMA ME & MICHIGAN CT			5.5	1.0		5.50			
152	ALABAMA AVE & FLORIDA AVE			10.5			10,50			
153	ALABAMA AVE & VERMONT CT		1	3.7	10.0	Sec.	37,00			
154	ALABAMA AVE @ BLDG. 5261			3.0		1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	30,00			
155	COLORADO AVE & DELAWARE AVE			2.7			27.00			
156	MICHIGAN AVE & COLORADO AVE			1.75	Part and		17.50			
157	MICHIGAN AVE & FLORIDA AVE			3.85			38,50			
158	MARYLAND AVE EVIRGINIA ST			8.05			80.50			
159	STONE ST @ HIGH SCHOOL			1.70			17,00			
160	VIRGINIA ST & LOUISIANA ST			5.0			50,000			
101	OREGON ST & FLORIDA AVE			2.5			25,00			
162	DELAWARE AVE & MARYLAND AVE			1.85			18,50			
163	DELAWARE AVE & FLORIDA AVE			9.50			95,00			
164	MARINE CORPS EXCHANGE			4.7		de sta	47,000			
65	VIRGINIA ST & BICENTENNIAL AVE			19			19,000			
66	BICENTENNIAL AVE & FLORIDA AVE			4.9		-	49,00			
67	HAWAII ST & NEBRASKA CT	-		4.1	1	1	41,000			
i sering										
		entre :								



TITL	E: CATHODIC PROTECTION SURVEY,	MAR	INE C	ORPS BAS	SE, C	AMP	LEJEU	NE, N.C.
SOIL	RESISTIVITY MEASUREMENTS	der.					<u> </u>	and the second
STR	UCTURE: MIDWAY PARK A	RE	<u>A9</u>					
DATE	E 11/8/84 ENGINEER CM/JH		TA	BLE I	-	PAG	DE _	OF 31
TEST NO.	TEST LOCATION	AVERAGE DEPTH		READING	MULTI.		FACTO	DR OHM-CM
170	BUTLER DR. GOUTH & BUTLER CT	5	-3"	1.9	10	7.0	1000	19 00
171	BUTLER DR. NORTH	•	20.0	5.65				56.50
172				2.9				29,00
173	4			2.25			49	22,50
174	BUTLER DR. NORTH @ WATER TWR			6.1				61,00
175	SECOND ST			2.55		-	e Se sa sa	25,50
176	BUTLER DR. NORTH			1.1	10	7.0		110,00
177	BUTLER DR. SOUTH			10.5	1	.0		10,50
178	BUTLER DR. GOUTH @ FIRE STA.			1.7	10	.0		17,00
179	BUTLER DR. SOUTH @ BLDG 1054			5.7			1	57,00
180	BUTLER DR. SOUTH @ BLDG 1500	-		6.4		-	-	64,00
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TITL	E: CATHODIC PROTECTION SURVEY	MAR	INE C	ORPS BAS	SE,C	AMP	LEJ	EUNE	, N.C.	
SOIL	RESISTIVITY MEASUREMENTS								Carl States	
STR	UCTURE: TARAWA TERRAC	EI	, A	REA IC	>			i i d	Carlos and	
DAT	E 11/8/84 ENGINEER NE/	56	TA	BLE I	-	PA	BE 1	20	DF <u>31</u>	
TEST NO.	TEST LOCATION	AVE	RAGE	READING	NG MULTI		FACTOR		онм-см	
190	TARAWA TERRACE ENTRANCE Nº, I	5	-3"	5.3	10	.0	10	20	53,000	
91	NORTH-WEST END OF ATHLETIC FIELD			5.8					58,000	
92	TARAWA TERRACE ENTRANCE Nº. 1			7.4	1.	0		1	7.400	
		10	16"	4.9			20	00	9,800	
		15	!9"	4:5			30	00	13,500	
		21	- 0"	4.0	1		40	00	16,000	
93	EAST PELELIU @ BLOG 960	5	- 3"	3.4	10.	0	100	00	34,000	
94	OROTE PL. @ BLDG 1628			2.7					27,000	
95	OROTE PL. @ CIRCLE		-	4.4					44,000	
96	EAST PELELIU @ BLDG. 1026			2,8				1.20	28,00	
97	WEST PELELIU @ BLDG. 1058			2.4			d.		24,000	
98	WEST PELELIU @ BLDG 1108			2.7	1				27,000	
99	EURIBACHI PL. CBLDG 1127	1.0		1.2	100	.0			120,000	
200	TARAWA BLVD. @ BLDG 21	1		2.2	10	0	~	-	22,000	
		10'	6"	10.8	1.	0	200	0	21,600	
		15-	9"	5.5			300	0	16,500	
		21-	0"	4.9	-		400	0	19,600	
01	WEST PELELIU @ BLDG. 599	5	3"	3.4	10.	0	100	Ø	34,000	
.02	WEST PELELIN @ BLDG. 481			4.8					48,000	
03	WEST PELELIU & BLDG. 369		· ·	1.9					19,000	
04	TARAWA BLVD & EAST PELELIU			1.6		- And	n sage of		16,000	
05	CAURT @ BLDG 1261			3.1	L			-	31,000	


SOIL RESISTIVITY MEASUREMENTS STRUCTURE: TARAWA TERPACE I, ARI DATE 11/8/84 ENGINEER NE/66 TABLE TEST INCHON 5T @ BLDG 1294 5'-3" 1.5 206 INCHON 5T @ BLDG 1294 5'-3" 1.5 207 INCHON 5T @ BLDG 1367 2.4 208 TINIAN ROAD 3.0 209 INCHON 5T @ BLDG 830 3.0 210 NAHA DR @ BLDG 709 2.3 211 MATANIKAU @ BLDG 1522 3.0 11 MATANIKAU @ BLDG 1522 3.0 11 MATANIKAU 0 BLDG 1522 3.0 12 MATANIKAU 0 BLDG 1522 3.0 13 MATANIKAU 0 BLDG 1522 3.0 14 MATANIKAU 0 BLDG 1522 3.0 15 MATANIKAU 0 BLDG 1522 3.0 16 MATANIKAU 0 BLDG 1522 3.0 17 MATANIKAU 0 BLDG 1522 3.0 18 MATANIKAU 0 BLDG 1522 3.0 19 MATANIKAU 0 BLDG 1522 3.0 10 MATANIKAU 0	EA 10 I P/ ING MULTI 5 10.0 4 1 3 100.0 5 10.0	PAGE 3 OF ULTI. FACTOR 0 2.0 1000 1 2.0 1000 1 2.0 1000 1 2.0 1000 1 3.0 3 0.0 2 0.0 2 0.0 2	<u>31</u> DHM-CM 5,000 4,000 9,000 6,000 30,000
STRUCTURE: IARAMIA IERRACE I., ARI DATE II/B/B4 ENGINEER NE/GG TABLE TEST TEST LOCATION AVERAGE DEPTH READ 206 INCHON 5T @ BLDG 1294 5'-3" I.5 207 INCHON 5T @ BLDG 1367 2.4 208 TINIAN ROAD 3.0 209 INCHON 5T @ BLDG 830 3.0 209 INCHON 5T @ BLDG 709 2.3 210 NAHA DR @ BLDG 1522 3.0 211 MATANIKAU @ BLDG 1522 3.0 212 IMATANIKAU IMATANIKAU	ING MULTI 5 10.0 4 10 5 10.0 4 10.0 5 10.0 5 10.0	PAGE 13 OF ULTI. FACTOR 0 2.0 1000 1 2.0 1000 1 2.0 1000 1 3.0 2 3 0.0 2 3 0.0 2 3 0.0 2 3	<u>31</u> DHM-CM 5,000 4,000 9,000 30,000 36,000
DATE II/B/B4 ENGINEER NC/CO TABLE TEST NO. TEST LOCATION AVERAGE DEPTH READ 206 INCHON ST @ BLDS 1294 5'-3" 1.5 207 INCHON ST @ BLDS 1294 5'-3" 1.5 207 INCHON ST @ BLDS 1367 2.4 208 TINIAN ROAD 3.0 209 INCHON ST @ BLDG 330 3.0 209 INCHON ST @ BLDG 520 3.0 210 NAHA DR @ BLDG 1522 3.0 211 MATANIKAU @ BLDG 1522 3.0 211 MATANIKAU 1.5 211 MATANIKAU 1.5 211 MATANIKAU 1.5 212 3.0 213 1.5 214 1.5 215 1.5 216 1.5 217 1.5 218 1.5 219 1.5 210 1.5 211 1.5 212 1.5	ING MULTI 5 10.0 1 0.0 1 0.0 5 10.0 1 0.0 5 10.0	PAGE 12 OF ULTI. FACTOR (2.0 1000 1 2.0 1000 1 2.0 1000 1 3.0 2 3 0.0 2 3 0.0 2 3 0.0 2 3	31 DHM-CM 5,000 4,000 9,000 30,000 30,000
TEST NO. TEST LOCATION AVERAGE DEPTH READ 206 INCHON 6T @ BLDG 1294 5'-3" 1.5 207 INCHON 6T @ BLDG 1367 2.4 208 TINIAN ROAD 3.9 209 TINIAN ROAD 3.9 209 INCHON 5T @ BLDG 830 3.9 209 INCHON 5T @ BLDG 709 2.3 210 NAHA DR @ BLDG 709 2.3 211 MATANIKAU @ BLDG 1522 3.0 211 MATANIKAU @ BLDG 1522 3.0 211 MATANIKAU 1.5 210 NAHA 1.5 210 NAHA 1.5 211 MATANIKAU 1.5 212 3.0 3.0 213 1.5 3.0 214 1.5 1.5 215 1.5 1.5 210 1.5 1.5 211 1.5 1.5 212 1.5 1.5 213 1.5 1.5 214 1.5 1.5 215 1.5 1.5	ING MULTI 5 10.0 4 7 6 7 3 100.0 6 10.0	ULTI. FACTOR (2.0 1000 1 2.0 3 3 0.0 2 0.0 10 0.0	5,000 4,000 9,000 6,000 30,000
206 INCHON ST @ BLDG 1294 5'-3" 1.5 207 INCHON ST @ BLDG 1367 2.4 208 TINIAN ROAD 3.0 209 INCHON ST @ BLDG 830 3.0 209 INCHON ST @ BLDG 709 2.3 210 NAHA DR @ BLDG 709 2.3 211 MATANIKAU @ BLDG 1522 3.0 212 3.0 3.0 213 INGARANIKAU @ BLDG 1522 3.0 314 INGARANIKAU @ INGARANIKAU 1.0 315 INGARANIKAU 1.0 316 INGARANIKAU 1.0 317 INGARANIKAU 1.0 318 INGARANIKAU 1.0 319 INGARANIKAU 1.0 310 INGARANIKAU 1.0 310 INGARANIKAU 1.0 311 INGARANIKAU 1.0 311 INGARANIKAU <th>5 10.0 1 1 7 1 6 1 3 100.0 6 10.0</th> <th>2.0 1000 1 2 3 3 3 0.0 2 1.0 2</th> <th>5,000 4,000 9,000 6,000 30,000</th>	5 10.0 1 1 7 1 6 1 3 100.0 6 10.0	2.0 1000 1 2 3 3 3 0.0 2 1.0 2	5,000 4,000 9,000 6,000 30,000
207 INCHON ST @ BLDG 1367 2.4 208 TINIAN ROAD 3.0 209 INCHON ST @ BLDG 830 3.0 209 INCHON ST @ BLDG 709 2.3 210 NAHA DR @ BLDG 709 2.3 211 MATANIKAU @ BLDG 1522 3.0 209 INCHON ST @ BLDG 709 2.3 210 NAHA DR @ BLDG 1522 3.0 211 MATANIKAU @ BLDG 1522 3.0 211 MATANIKAU @ BLDG 1522 3.0 212 3.0 3.0 213 3.0 3.0 214 MATANIKAU @ BLDG 1522 3.0 215 3.0 3.0 211 MATANIKAU @ BLDG 1522 3.0 313 314 314 314 315 314 315 315 316 316 316 316 317 316 316 318 316 316 319 316 316 319 316 316 319 316 316	4 7 6 3 100.0 6 10.0		4,000 9,000 6,000 30,000
208 TINIAN ROAD 3.0 209 INCHON 51 @ BLDG 830 3.0 210 NAHA DR @ BLDG 709 2.3 211 MATANIKAU @ BLDG 1522 3.0 211 MATANIKAU @ BLDG 1522 3.0 212 3.0 3.0 213 MATANIKAU @ BLDG 1522 3.0 214 MATANIKAU @ BLDG 1522 3.0 215 3.0 3.0 216 1.0 1.0 217 MATANIKAU @ BLDG 1522 3.0 218 1.0 1.0 319 1.0 1.0 310 1.0 1.0 311 1.0 1.0 311 1.0 1.0 311 1.0 1.0 311 1.0 1.0 312 1.0 1.0 313 1.0 1.0 314 1.0 1.0 315 1.0 1.0 314 1.0 1.0 315 1.0 1.0 316 1.0 1.0	7 6 3 100.0 6 10.0		9,000 6,000 30,000 36,000
209 INCHON - T @ BLDG 830 3.0 210 NAHA DR @ BLDG 709 2.3 211 MATANIKAU @ BLDG 1522 3.0	6 3 100.0 6 10.0		6,000 30,000 36,000
210 NAHA DR @ BLDG 709 2.3 211 MATANIKAU @ BLDG 1522 3.0 	3 100.0 6 10.0		30, 000 36, 000
211 MATANIKAU @ BLDG 1522 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0	6 10.0	n.o 🖌 🗄	36,000
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TITL	E: CATHODIC PROTECTION SURVEY,	MARI	NE C	ORPS BAS	SE,C	AMP	LEJ	EUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS	1		No. And					
STR	UCTURE: TARAWA TERRACE	I,	A	REA 11		ing the			
DATI	E 11/8/84 ENGINEER NE/G	6	TA	BLE I	19 ¹⁵ 81	PAG	BE	4	DF <u>31</u>
TEST NO.	TEST LOCATION	AVE DE	RAGE PTH	READING	MU	LTI.	FA	CTOR	ОНМ-СМ
220	IWO JIMA @ENTRANCE	5	!3"	7.5	12	0.0	10	00	75,000
221	IWO JIMA BLYD		,	2.2			1		22,000
		10!	6"	1.3		L	20	100	26,000
		15!	9"	10.2		.0	30	00	30,600
-		21	-0"	8.9			40	00	35,600
222	\downarrow	5	:3"	8.3			10	00	8,300
223	INO JIMA BLVD & INCHON ST			8.4					8,400
224	L E TARAWA			6.4					6,400
225	TARAWA BLVD			9.2	1				9,200
226	ROAD TO SEWAGE DISPOSAL		,	4.3	10	0	1	1	43,000
i de la color a		10!	6"	2.4			200	20	48,000
		15'	9"	2.5			30	00	75,000
		21-	0"	1.4			400	20	56,000
227	HAGARU DR. @ BLOG 3385	5!:	3"	2.2			100	00	22,000
228	CHOSIN CIRCLE @BLDG 3544			2.0	-	-			20,000
229	GUAM AVE & AGANA PL.			8.1	1.0	2	11.201	and a	8,100
230	BOUGAINVILLE DR.	-	-	1.3	10,	0		1	13,000
		10-	6"	4.9	1.0	2	20	20	9,800
		15-	<u>a" </u>	4.1		_	30	00	12,300
201	↓ ↓	21-	0"	3.0	-	-	400	20	12,000
231	BOUGAINVILLE & BLDG. 3140	5-	3"	1.5	10.	0	100	00	15,000
232	BOUGAINVILLE & TARAWA BLVD		-	1.6	1	•	1	-	16,000



SOIL RESISTIVITY MEASUREMENTS STRUCTURE: TARAWA TERRACE I, AREA II DATE II/0/04 ENGINEER NE/05 TABLE I PAGE 15 OF 31 TEST ICATION AVERAGE DEPTH READING MULTI. FACTOR OHM- 233 BOUGAINVILLE DR @ BLOG 2631 5'.3" 5.2 10.0 1000 52, 234 BOUGAINVILLE DR @ BLOG 2631 5'.3" 5.2 10.0 1000 52, 235 BUGAINVILLE DR @ BLOG 2631 5'.3" 5.2 10.0 20, 7,1 235 BUGAINVILLE DR @ BLOG 2631 2'.0 10.0 20, 20, 20, 236 BUGAINVILLE DR @ HATER TANK 6.7 1 6,1 21, 21, 237 OPEN 'FIELD @ TARAWA \$6CHOOL 2.1 2.1 21, 21, 21, 238 TARAWIA BLVD @ BLOG 30.19 2.0 1.4 14, 24, 239 SAIPAN DR @ BLOG 30.19 2.0 1.4 14, 24, 239 SAIPAN DR @ BLOG 30.19 2.0 1.4 1.4 1.4 230 <td< th=""><th>SOLL RESISTIVITY MEASUREMENTS STRUCTURE: TARAWA TERRACE I AREA II DATE II/0/044 ENGINEER NE/05 TABLE I PAGE 5 OF 31 TEST LOCATION AVERAGE DEPTH READING MULTI. FACTOR OHM-C 233 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52,0 234 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52,0 235 BOUGAINVILLE DR @ AIPAN DR 7.1 1.0 7,10 35 BOUGAINVILLE DR @ AIPAN DR 7.1 1.0 7,10 35 BOUGAINVILLE DR @ BLDG 2811 2.0 10.0 20,0 237 OPEN FIELD @ TARAWA \$GOLOL 2.1 21,0 238 TARAWA BUXD @ BLDG 202 1.4 14,0 249 SAIPAN DR @ BLDG 3019 2.0 2 20,0 24 25 25 25 25 25 25 25 25 25 25 25 25 25</th><th>TITL</th><th>E: CATHODIC PROTECTION SURVEY,</th><th>MARINE C</th><th>ORPS BAS</th><th>SE, CAMP</th><th>LEJEUNE</th><th>, N.C.</th></td<>	SOLL RESISTIVITY MEASUREMENTS STRUCTURE: TARAWA TERRACE I AREA II DATE II/0/044 ENGINEER NE/05 TABLE I PAGE 5 OF 31 TEST LOCATION AVERAGE DEPTH READING MULTI. FACTOR OHM-C 233 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52,0 234 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52,0 235 BOUGAINVILLE DR @ AIPAN DR 7.1 1.0 7,10 35 BOUGAINVILLE DR @ AIPAN DR 7.1 1.0 7,10 35 BOUGAINVILLE DR @ BLDG 2811 2.0 10.0 20,0 237 OPEN FIELD @ TARAWA \$GOLOL 2.1 21,0 238 TARAWA BUXD @ BLDG 202 1.4 14,0 249 SAIPAN DR @ BLDG 3019 2.0 2 20,0 24 25 25 25 25 25 25 25 25 25 25 25 25 25	TITL	E: CATHODIC PROTECTION SURVEY,	MARINE C	ORPS BAS	SE, CAMP	LEJEUNE	, N.C.
STRUCTURE: $ AKAAN ERCACE I. AKEA II DATE II/B/B4 ENGINEER NE/GG TABLE I PAGE IS OF 31 TEST LOCATION AVERAGE DEPTH READING MULTI. FACTOR OHM- 2332 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52, 234 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 20, 234 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 7,1 234 BOUGAINVILLE DR @ BLDG 2811 2.0 7 235 BOUGAINVILLE DR @ BLDG 2811 2.0 20, 237 OPEN FIELD @ TARAWA & GCHOOL 2.1 2 238 TARAWA BLD@ @ BLDG 3019 2.0 2 239 20 2 2 20 2 20 2 20 2 20 2 $	STRUCTORE: [ARANA]ERCACE I, AREA II DATE]]/ $B/B4$ ENGINEER NE/GG TABLE I PAGE S OF 31 TEST NO. TEST LOCATION AVERAGE DEPTH READING MULTI. FACTOR OHM-C 233 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52, 0 234 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52, 0 235 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52, 0 236 BOUGAINVILLE DR @ BLDG 2631 2.0 10.0 20, 0 237 OPEN FIELD @ TARANA \$6CHOOL 2.1 2.0 10.0 20, 0 238 TARAWA BLDD @ BLDG 202 1.4 14, 0 24 GAIPAN DR @ BLDG 3019 2.0 10.0 20, 0 25 DELGAINVILLE DR @ BLDG 3019 2.0 10.0 20, 0 26 DELGAINVILLE DR @ BLDG 3019 2.0 10.0 20, 0 27 OPEN FIELD @ TARANA \$6CHOOL 2.1 2.1 2.0 2.0 2.0 2.0 0 28 TARAWA BLDD @ BLDG 3019 2.0 10.0 20, 0 29 SAIPAN DR @ BLDG 3019 2.0 10.0 20, 0 20, 0 20	SOIL	RESISTIVITY MEASUREMENTS	T 1.				
DATEIIVE/CAPENGINEERNUCCSTABLEPAGEPAGEIOFTESTTEST LOCATIONAVERAGE DEPTHREADINGMULTI.FACTOR0HM-233BOUGAINVILLEDR $0HB$ $5'.3''$ 5.2 10.0 1000 52 234BOUGAINVILLEDR $0HB$ 7.1 1.0 7.1 0.0 52 235BOUGAINVILLEDR $0HB$ 7.1 1.0 7.1 0.0 20 236BOUGAINVILLEDR $0HB$ 2.1 2.0 10.0 20 237OPEN FIELDCTARAWA $$0CAOL$ 2.1 2.1 2.1 21 238TARAWA BUDCBLDG 2002 1.44 144 239SAIPANDRBLDG 3019 2.0 2.0 20 239SAIPAN DRBLDG 3019 2.0 2.0 20 239SAIPAN DRBLDG 3019 2.0 2.0 2.0 230SAIPAN DRBLDG 3019 2.0 2.0 2.0 231SAIPANDRBLDG 3019 2.0 2.0 2.0 232SAIPANDRBLDG 3019 2.0 2.0 2.0 233SAIPANDRBLDG 3019 2.0 2.0 2.0 234SAIPANDR 2.0 2.0 2.0 2.0 235SAIPANSAIPAN 2.0 2.0 2.0 236SAIPA	DATE INDEX ENGINEER ILZ/SE TABLE Image is a straight of the straigh	DATE	ULA AL ENGINEER NE C	$\frac{1}{2}$, AF	KEA II		- 15 -	- 01
TEST TEST LOCATION AVERAGE DEPTH READING MULTI. FACTOR OHM- 233 BOUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52, 234 BUGAINVILLE DR @ BLDG 2631 5'.3" 5.2 10.0 1000 52, 235 BUGAINVILLE DR @ GAIPAN DR 7.1 1.0 7,1 6, 236 BUGAINVILLE DR @ MATER TANK 6.7 1 6, 236 BUGAINVILLE DR @ BLDG 2811 2.0 10.0 20, 237 OPEN FIELD @ TARAWA \$ 60400L 2.1 14, 14, 238 TARAWA BUD @ BLDG 2202 1,4 14, 14, 239 GAIPAN DR @ BLDG 3019 2.0 1 20, 239 CALPAN DR @ BLDG 3019 2.0 1 20, 230 231 231 231 231 231 231 232 232 234 232 234 233 DAPAN DR @ BLDG 3019 2.0 2.0 2 20, 234 234 234 234 234 234	TEST NO. TEST LOCATION AVERAGE DEPTH READING MULTI. FACTOR OHM-C 233 BOUGAINVILLE DR @ BLOG 2631 $5'.3''$ 5.2 10.0 1000 $52.a''$ 234 BOUGAINVILLE DR @ BLOG 2631 $5'.3''$ 5.2 10.0 1000 $52.a''$ 235 BOUGAINVILLE DR @ BLOG 2631 $5'.3''$ 5.2 10.0 1000 $52.a''$ 236 BOUGAINVILLE DR @ BLOG 26311 2.0 1.0 $7.1''$ $6.7''$ 36 BOUGAINVILLE DR @ BLOG 2811 2.0 10.0 $20.c''$ 237 OPEN FIELD @ TARAWA \dot{f} 6cHool $2.1'' 2.0''' 2.0''''''''''''''''''''''''''''''''''''$	DATE	ENGINEER NU/GO	41 2	ABLE			F <u>21</u>
2333 BOLGAINVILLE DR @ BLDG 2631 $5.3"$ 5.2 10.0 1200 52 , 234 BOLGAINVILLE DR @ GAIPAN DR 7.1 1.0 7,1 235 BOLGAINVILLE DR @ MATER TANK 6.7 \bot 6 , 236 BOLGAINVILLE DR @ BLDG 2811 2.0 10.0 20 , 236 BOLGAINVILLE DR @ BLDG 2811 2.0 10.0 20 , 237 OPEN FIELD @ TARANA & GCHOOL 2.1 2.1 2.2 , 238 TARAMA BUD @ BLDG 2202 1.4 14 , 14 , 239 SAIPAN DR @ BLDG 3019 2.0 2.0 20 , 239 SAIPAN DR @ BLDG 3019 2.0 2.0 20 , 239 SAIPAN DR @ BLDG 3019 2.0 2.0 2.0 2.0 239 SAIPAN DR @ BLDG 3019 2.0 2.0 2.0 2.0 2.0 230 2.0 2.0 2.0 2.0 2.0 2.0 230 2.0 2.0 2.0 2.0 2.0 2.0 2.0 231	233 BOUGAINVILLE DR @ BLDG 2631 $5!.3"$ 5.2 10.0 1000 $52, e$ 234 BOUGAINVILLE DR @ GAIPAN DR 7.1 1.0 $7, 1'$ 35 BOUGAINVILLE DR @ GAIPAN DR 7.1 1.0 $7, 1'$ 36 BOUGAINVILLE DR @ GAIPAN DR 6.7 10.0 $20, c$ 36 BOUGAINVILLE DR @ BLDG 2811 2.0 10.0 $20, c$ 37 DPEN FIELD @ TARANA \$ 60400L 2.1 2.1 $21, c$ 38 TARAMA BLAD @ BLDG 2202 1.4 $14, c$ $44, c$ 39 GAIPAN DR @ BLDG 3019 2.0 $20, c$ $20, c$ 39 GAIPAN DR @ BLDG 3019 2.0 2.0 $20, c$ 30 30.19 2.0 4 $20, c$ 30 30.19 2.0 4 $20, c$ 31 30.19 2.0 4 $20, c$ 31 30.19 30.19 30.19 30.19 30.19 31 30.19 30.19 30.19 30.19 30.19 30.19	TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	ОНМ-СМ
234 BOLGAINVILLE DR & GAIPAN DR 7.1 1.0 7,1 235 BOLGAINVILLE DR & GAIPAN DR 6.7 \downarrow 6, 236 BOLGAINVILLE DR & BLDG 2811 2.0 10.0 20, 237 OPEN FIELD & TARAWA & SCHOOL 2.1 21, 21, 238 TARAWA BLVD & BLDG 2202 1.4 14, 14, 239 SAIPAN DR & BLDG 3019 2.0 20, 20, 239 SAIPAN DR & BLDG 3019 2.0 20, 20, 239 SAIPAN DR & BLDG 3019 2.0 20, 20, 239 SAIPAN DR & BLDG 3019 2.0 20, 20, 239 SAIPAN DR & BLDG 3019 2.0 20, 20, 230 230 230, 230, 20, 20, 231 30, 30, 30, 30, 30, 30, 331 31, 31, 31, 31, 31, 31, 331 31, 31, 31, 31, 31, 31, 31, 331 31, 31,	2234 BUGAINVILLE DR $($ GAIPAN DR 7.1 1.0 7.1 35 BUGAINVILLE DR $($ WATERTANK 6.7 6.7 6.7 36 BUGAINVILLE DR $($ BLDG 2811 2.0 10.0 20,0 37 OPAN FIELD $($ TARAWA $($ Goldon L 2.1 21,0 21,0 38 TARAWA BUD $($ BLDG 2202 1.4 14,0 14,0 39 SAIPAN DR $($ BLDG 3019 2.0 20,0 20,0 39 SAIPAN DR $($ BLDG 3019 2.0 20,0 20,0 30 SAIPAN DR $($ BLDG 3019 2.0 20,0 20,0 30 SAIPAN DR $($ BLDG 3019 2.0 20,0 20,0 31 SAIPAN DR $($ BLDG 3019 2.0 20,0 20,0 32 SAIPAN DR $($ BLDG 3019 2.0 20,0 20,0 33 SAIPAN DR $($ BLDG 3019 2.0 2.0 20,0 34 SAIPAN DR $($ SA	233	BOUGAINVILLE DR @ BLDG 2631	5'-3"	5.2	10.0	1000	52.a
235 Balgan NVILLE DR @ HATERTANK 6.7 6.7 236 Balgan NVILLE DR @ BLDG 2811 2.0 10.0 20, 237 OPEN FIELD @ TARAWA \$ 5CHOOL 2.1 21, 21, 238 TARAWA BUD @ BLDG 2202 1.4 14, 14, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 230 3.1 3.1 3.1 3.1 331 3.1 3.1 3.1 3.1 3.1 331 3.1 3.1 3.1 3.1 3.1 331 3.1 3.1 3.1 3.1 3.1 331 3.1 3.1 3.1 3.1 3.1 331 3.1 3.1 <td>35 BUGAN VILLE DR @ HATER TANK 6.7 6.7 36 BUGAN VILLE DR @ BLB2 2811 2.0 10.0 20,0 37 OPEN'FIELD @ TARAWA & GOHOOL 2.1 21,0 21,0 38 TARAWA BLVD @ BLDG 2202 1.4 14,0 14,0 39 SAIPAN DR @ BLDG 3019 2.0 20,0 39 SAIPAN DR @ BLDG 3019 2.0 20,0 30 30 3019 2.0 20,0 30 3019 3019 3019 3019 3019 30 3019 3019 3019 3019 3019 3019 30 3019 3019 3019 3019 3019 3019 30 3019 3019 3019 3019 3019 3019 30 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3</td> <td>234</td> <td>BOUGAINVILLE DR & GAIPAN DR</td> <td></td> <td>7.1</td> <td>1.0</td> <td></td> <td>7,10</td>	35 BUGAN VILLE DR @ HATER TANK 6.7 6.7 36 BUGAN VILLE DR @ BLB2 2811 2.0 10.0 20,0 37 OPEN'FIELD @ TARAWA & GOHOOL 2.1 21,0 21,0 38 TARAWA BLVD @ BLDG 2202 1.4 14,0 14,0 39 SAIPAN DR @ BLDG 3019 2.0 20,0 39 SAIPAN DR @ BLDG 3019 2.0 20,0 30 30 3019 2.0 20,0 30 3019 3019 3019 3019 3019 30 3019 3019 3019 3019 3019 3019 30 3019 3019 3019 3019 3019 3019 30 3019 3019 3019 3019 3019 3019 30 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3	234	BOUGAINVILLE DR & GAIPAN DR		7.1	1.0		7,10
236 BOLBAINVILLE PR @ BLDG 2811 2.0 10.0 20, 237 OPEN FIELD @ TARAWA \$ 50400L 2.1 2.1, 21, 238 TARAWA BLVD @ BLDG 2202 1.4 14, 14, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 230 20, 20, 20, 20, 231 20, 20, 20, 20, 239 SAIPAN DR @ BLDG 3019 2.0 20, 20, 230 20, 20, 20, 20, 20, 230 20, 20, 20, 20, 20, 231, 20, 20, 20, 20, 20, 231, 21, 21, 20, 20, 20, 20, 232, 233, 24, 24, 24, 24, 24, 24, 233, 24, 24, 24, 24, 24, 24, 24, 233, 24, 24, 24, 24,	36 BULSAINVILLE DR @ BLDG 2811 2.0 10.0 20,0 37 OPAN FIELD @ TARAWA \$ 5000L 2.1 21,0 21,0 38 TARAWA BUD @ BLDG 2202 1.4 14,0 14,0 39 SAIPAN DR @ BLDG 3019 2.0 20,0 20,0 39 SAIPAN DR @ BLDG 3019 2.0 20,0 20,0 30 3019 2.0 20,0 3019 30 3019 3019 3019 3019 31 3106 3019 3019 3019 310 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019 3019	235	BUGANNULLE DR @ WATERTANK		6.7			6,70
237 OPEN FIELD @ TARAWA \$ 5 CHOOL 2.1 21, 238 TARAWA BLVD @ BLDG 2202 1,4 14, 239 SAIPAN DR @ BLDG 3019 2.0 20, 230 20, 20, 20, 230 20, 20, 20, 230 20, 20, 20, 230 20, 20, 20, 230 20, 20, 20, 231 20, 20, 20, 232 20, 20, 20, 233 20, 20, 20, 233 20, 20, 20, 233 20, 20, 20, 234 20,	137 OPEN FIELD @ TARAWA \$ ECHOOL 2.1 21,0 138 TARAWA BUD @ BUDG 2202 1.4 14,0 24 GAIPAN DR @ BUDG 3019 2.0 20,0	236	BOLGAINVILLE DR @ BLOG 2811		2.0	10.0		20,00
238 TARAWA BUND @ BUDG 2202 1,4 14, 239 SAIPAN DR @ BUDG 3019 2.0 20, 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1.4 14, 39 SAIPAN DR @ BLDG 3019 2.0 20, 2.0 20, 20, 20,	237	OPEN FIELD & TARAWA & SCHOOL		2.1			21,00
		238	TARAWA BLVD @ BLDG 2202		1.4			14,00
		239	SAIPAN DR @ BLDG 3019	+	2.0	+	-	20,00
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TITL	E: CATHODIC PROTECTION SURVEY,	MARINE C	ORPS BAS	E, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS	<u></u>			1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	
STRU	JCTURE: TRAILER PARK ARE	EA 12				Section .
DATE	E 11/8/84 ENGINEER CM/ J	<u>. т</u> а	BLE I	PAG	E 16 0	DF <u>31</u>
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	ОНМ-СМ
250	CAMP KNOX RD	5-3"	2.8	10.0	1000	28,000
251			1.7			17.000
252	eichmonip rd		8.3	1.0		8,300
253	TOLEDO RD		1.4	10.0		4.000
254	BALTIMORE RD		9.6	1.0		9,600
255	BOSTON & NORFOLK CT		7.4			7,40
256	FLORENCE & DETROIT CT		7.1			7,100
257	BUFFALO CT		7.0	-		7,000
258	CAMP KNOX & FLORENCE RD		1.2	10.0		12,000
259	BOSTON RD	-	5.2	1.0	-	5,200
		-				1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 - 1994 -
		1			1	<u>,</u>
						25 - 10 1
			and the second			
		1.5				
			183			1
		2			No.	
	and the second					



TITL	E: CATHODIC PROTECTION SURVEY,	MAR	NE C	ORPS BAS	SE, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS			A Startes			1. 18 (31)
STR	JCTURE: CAMP KNOX A	RE4	4 13	3		A series	
DATE	II/8/84 ENGINEER CM/JH	ł	T	ABLE I	PAG	E 17	DF 31
TEST NO.	TEST LOCATION	AVE DE	RAGE PTH	READING	MULTI.	FACTOR	онм-см
260	RALEIGH RD	5	-31	1.1	10.0	1000	11.000
261	AUSTIN DR			7.5	1.0		7500
262	LEXINGTON CT			1.1	10.0		11.00
263	RALEIGH RD & DENVER CT			4.7	1.0		4,700
264	SACREMENTO RD		/	4.8	1		4,80
		7 7 1					
		÷					1200
							A.S.
			21.5				
	и 						
			-+				· ****
Al. 199			-+			2	
					Sec. in		1.25
					1		
					11.15.45.45 ²		
1	and the second se	1.000				·	



TITL	E: CATHODIC PROTECTION SURVEY,	MAR	INE C	ORPS BAS	SE, CAMP	LEJ	EUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS	- Aller				1911 A 19		
STRU	JCTURE: MONTFORD POINT A	RE	A 14	•				and the second
DATE	11/9/84 ENGINEER CM/J	H	TA	BLE I	PA	GE _	18 0	of <u>31</u>
TEST NO.	TEST LOCATION	AVE	RAGE	READING	MULTI.	F/	ACTOR	ОНМ-СМ
270	MONTFORP LANDING RD	5	1-3"	1.6	10.0	10	00	16,000
271	NEW CONSTRUCTION @ BLDG 131		1	1.7			ŀ	17.000
272	HARLEM DR @ BLPG 415		4	10.9	.1			109,000
273	CO. ST. 'D' @ BLDS 622			1.4	1.0			1,400
274	CO. ST 'E' C BLOG 518		Sec. 1	7.6			1.00	7,600
275	FLORENCE ROCELOG E.I			3.5				3,500
276	NEUSE RD @ BLOG 105	and a second		2.5	\downarrow			2,500
277	MONTFORD LANDING RD @ 142			1.1	10.0			11,000
278	WASTE TREATMENT PLANT			1.5	1.0			1,500
279	WILSON @ TENNIS COURTS		1.00	2.3	10.0	No.		23,000
280	HOOVER RD @ BLDG 203			6.9	10.0			69,000
281	END OF HAYES ST	,	-	6.5	1.0	,		6,500
		- 			r			:
						a		ì
19. 1								1
			-					
		-						i serai e re

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SOIL	RESISTIVITY MEASUREMENTS					
STR	UCTURE: CAMP GEIBER	AREA	15			
DATE	= 11/9/84 ENGINEER CM/J	<u>н</u> т	ABLE I	PA0	BE 19	OF 31
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	онм-с
290	ELEVENTH & 'A' ST	5:3"	2.7	10.0	1000	27.00
291	BLPG TC 1063		8.5	1.0		850
292	9 I ST @ BLDG TC 853		9.7			9,70
293	C'ST @ BLDG TC 1021		7.3			7.3
294	C'ST @ BLDG TO 821		5.0			5.0
295	'A' ST @ BLDG TE 910		3.5	10.0		35,00
296	7 H ST @ BLDG TC. 704	1	8.0			80,0
297	GHST@BLDG G 551		1.1			11,0
298	は好きろ型ケ		1.2			12,0
299	に町も何町		9.7	1.0		9,7
300	GIST @ BLOG TC 474		1.9			.10
301	BRINSON CREEK		8.4			84
302	'G'ST @ BLDG TC 609		4.9			4.9
303	CURTISS RD @ SUBSTATION		1.5	10.0		15,0
304		\downarrow	8.0	1.0		8,0
			;		<u>. 40</u> 2. 	
				-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	1 de la	
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	in the second		A State			



	E. CATHODIC PROTECTION SURVEY,	MAR	INE C	ORPS BAS	SE, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS		ine -				
STRU	CTURE: GEIGER TRAIL	ER	PA	RK A	REAI	6	a des stad
DATE	11/9/84 ENGINEER CM/JH	<u> </u>	TA	BLE I	PAG	E 20 0	F 31
TEST NO. '	TEST LOCATION	AVE	RAGE PTH	READING	MULTI.	FACTOR	ОНМ-СМ
310	R.L. WILSON BLVD & BORDELON ST	5	-3"	4,5	1.0	1000	4.500
311	BORDELON ST		Î.	2.5	10.0		1500
312	AGERHOLM ST	1.00	10.000 0.000	1.2		1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 -	12.000
313	R.L. WILSON BLVD & EPPERSON ST	1.4	1.00	3.8	1.0		3,800
314	EPPERSON ST & HAWKINS BLVD			1.5	10.0		15.000
315	HAWKING BLVD			1.7			17:000
016	R.L., WILSON BLVD			1.5			15,00
317		1		6.3	1.0		6,30
							A. South -
-							
				and the second second			
							;
				The second second			
			1				



TITL	E: CATHODIC	PROTECTION SURVEY	, MA	RINE C	ORPS B	ASE,	CAMF	LE	JEUN	E, N.C.
SOIL	RESISTIVITY	MEASUREMENTS							en e	1996
STR	UCTURE: K	FLE RANGE	AR	EA	17	<u></u>	<u>Grande</u>		<u></u>	. The second of the
DATE	= 11/6/84	ENGINEER NE	66	T	ABLE]	_	PA	GE	21	OF 31
TEST NO.	TEST	LOCATION	AV	VERAGE DEPTH	READIN	G M	ULTI.	F	ACTOR	ОНМ-СМ
320	RANGE	RD .	. 5	5!3"	4.4	1	0.0	10	000	44.00
			10	51-6"	1.6	-		2	000	30.00
			15	5'-9"	10.4		1.0	3	000	31 20
10		• · · · · · · · · · · · · · · · · · · ·	2	1-0"	1.2	10	0.0	40	200	48.00
321	PONDER	LN	5	51-31	4.1		1	10	000	41.000
322	POWDER	LN C BLDG 72	1.	\downarrow	4.3		1000		L	43,00
			10	16"	1.3			20	000	26,00
323	PONDER	N @ BLDG 15	5	23"	30.0		and the second	10	000	300,000
324	ROAD OFF	range RD			2.4		1.0	-		24.000
			10	-6"	1.5			20	∞	30,000
			15	:q"	1.1	-		30	000	33,000
		<u> </u>	21	-0	9.5	1	0	40	000	38,00
325	RANGE RE	7	5	5.3"	19.0	10	0	10	00	190,000
326					23.0	5	-		-	230,000
327	2000 ·				7.5	1.	0		·	7,500
328	V	· @ BLDG 69			1.0	10.	0	1		10,000
229	BOOKER I. H	iashing on BLVD			1.0					10,000
221					29					29,000
200	G.W.CARVER	ST. @ BLDG 212			1.1					11,000
196	KUAD OFF	kange KD	-		9.2	~		1	2	92,000
			10-	01	5.0	1.0		20	00	10,000
			-'9'-	9"	2.7			300	20	8,100
			21-	0'	1.8	\checkmark	- 1.	400	0	7,200



TITL	E: CATHODIC PROTECTION SURVEY,	MARIN	EC	ORPS BA	SE, CA	AMP	LEJ	EUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS			A 10		<u>.</u>			
STR	UCTURE: COURT HOUSE BA	r Al	ZE	AIB				1	
DAT	E <u>11/6/84</u> ENGINEER <u>NE/G</u>	G	TA	BLE I		PA	BE 2	22 0	of <u>31</u>
TEST NO.	TEST LOCATION	AVER/ DEPT	NGE TH	READING	MUL	ті.	FA	CTOR	онм-см
340	COURTHOUSE RD	5!.	3"	9,4	1.0	0	10	00	940
		10-	5"	4.6	1				4,600
		154	9"	3.1					3,100
	~	21-	0"	2.7					2,700
341	COURTHOUSE RD@ U.G. FUELTANKS	5'-	3"	9.1					9,100
		10-6	;"	4.7					4,700
		1512	71	3,2			-91		3,20
	4	21-0	>"	2.8	1.1.1.1				2,80
342	COUIZTHOUSE RD	5-:	5"	2.5					2,50
343	SNEADS FERRY RD			2.9		-			2,90
344				2.2	100.	0			220,0
345	MARINES RD			6.5	1.0	2			6,50
346	BERDC BLDG 71			7.5			-	-	7,50
		10'-6	"	4.0			200	0	8,00
L		15-0	n	2,4		_	30	00	7;20
		21-0	»"	1.5	1	•	400	20	6,00
341	KOAU & BLUG 50	5-3	"	1.9	10.0	2	100	0	19,:00
348	POE RD			1.8					18,00
349	MARINES RD		-	3.4				_	34,00
250	DEACH	-	-	7.9					79,00
271	FLACH ST.		+	3.5		-			35,00
352	ELLEN PATH		+	1,4	4	-	1	-	14,00
				100					

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TITL	E: CATHODIC PROTECTION SURVEY,	MARINE	ORPS BA	SE, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS			Salar I	Bergh	Service and
STR	JCTURE: COURT HOUSE BA	y Ar	EA IB		AND DES A STAT	
DATE	11/6/84 ENGINEER NE/G	G T	ABLE I	PA	GE 23	OF 31
TEST NO. Y	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	ОНМ-СМ
353	FRONT ST	5'-3"	1.2	10.0	1000	12,000
354			2.3	1.0	La Land	2,300
355	CLINTON ST		1.3	10.0		13,000
356	MARINES RD		1.4			14,000
357	MARINES RD @ GAS STATION		3.1			31,000
		10'-6"	1.6		2000	32,000
-		15'-9"	8.4	1.0	3000	25,200
	\checkmark	21-0"	6.0	-	4000	24,000
358	GRAGE LN @ BLPG BO	5'-3	2.1	10.0	1000	21,000
359	SNEADS PERRY & MARINE RD		3.15	100.0	+	315,000
		10-6"	1.1	\downarrow	2000	220,000
		15-9"	5.65	10.0	3000	169,500
	4	21-0"	3,35	-	4000	134,000
						· · · ·
						· · ·
-					<u>.</u>	
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	and a second					an a ta a

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TITL	E: CATHODIC PROTECTION SURVEY	MARINE	CORPS BAS	SE, CAMP	LEJEUNE	. N.C.
SOIL	RESISTIVITY MEASUREMENTS		and a l			A. 17
STR	UCTURE: ONSLOW BEAC	H AR	EA 19			
DATE	E 11/9/84 ENGINEER Ch	<u>1.</u> т	ABLE I	PA	GE <u>24</u>	OF 31
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	OHM-0
370	ACCESS RD. AT TURNAROUND	5-3	3.10	10	1000	31,0
		10-6	1.30	10	2000	26,0
		15-91	8.10	1	3000	24.
Sec. 1	\downarrow	21-0	1.10	1	4000	28,4
371	BEACH AVE. AT BEACH RD.	5-31	1.15	100	1000	115,0
		10-6"	2.60	10	2000	52,0
		15-91	6.5	1	3000	19,5
on di	4	21-01	1.20	1	4000	4,80
372	BEACH AREA FRONT BA-115	5-31	4.85	10	1000	48,50
		10-61	1.95	10	2000	39,0
		15-9"	1.40	10	3000	42,0
	↓ · · · · · · · · · · · · · · · · · · ·	21-01	1.90	10	4000	76,00
373	BEACH AREA AT WATER TANK	5-3	1.70	10	1000	17,0
		10-6"	7.0	1	2000	14,00
1		15-91	9.0	1	3000	27,0
	¥	21-01	5.1	1	4000	20,4
374	BEACH AREA AT BA-105	5-31	2.60	10	1000	26,00
		10-6	1.20	10	2000	24,00
		15-9	1.05	10	3000	31,50
0	· V	21-0	6.60		4000	26,40
375	TUEL TANK AT BLDG. BA-106	5-3	4.3	1	1000	4,30
		10-6"	1.4		2000	2,80
376	FUEL TANK AT BLOG BA-106	5-31	1.0	1	000	1,00
•	\downarrow	10-61	1.1		2000	2.200



TITL	E: CATHODIC PROTECTION SURVEY,	MAR	NE C	ORPS BAS	SE, CAMP	LEJEUN	E, N.C.
SOIL	RESISTIVITY MEASUREMENTS					40	
STRU	ICTURE: FRENCH CREEK	AR	EA	20	- Adapt		
DATE	11/9/84 ENGINEER CM	-	T	BLE I	PA	GE 25	OF 31
TEST NO.	TEST LOCATION	AVE	RAGE PTH	READING	MULTI.	FACTOR	R OHM-CM
390	MAIN SERVICE RD AT CREEK	5	-31	6.90	1	1000	6,900
391	" " AT DALY ST.	1.		1.35	10		13.500
392	1 1 AT BLDG. ES-200			6.40	10		64.000
393	AT WATER TOWER			1:20	100		120,000
394	BLPG. FC-566			2.50	10	A. Arab	25.00
395	GONZALEZ BLVD. AT REASONER ST.			3.50	10	- and	35.000
396	II II ANDERSON ST			4.90	10		49,000
391	ANDERSON ST. AT BARKER RD.			1.90	10	and the second	19,000
398	AT BLDG. FE. 202	1	/	5.4	10		54.000
	(UNDERGROUND TANK)	101	6"	3.3	10	2000	66,000
				State of		a dha phra	
,							,
				1		48	
			100	petri -			
		1			1.1	1.4	1
				1			
							<u> </u>
· ·	and the second		-			10 A	· · · · ·
							-
					+ 1		



TITL	E: CATHODIC PROTECTION SURVEY,	MARINE	CORPS BAS	E, CAMP	LEJEUNE	, N.C.
SOIL	RESISTIVITY MEASUREMENTS	Aliange de			<u></u>	40.00
STRU	ICTURE: NEW NAVAL HOSPI	TAL	AREA	21	al Bran	
DATE	11/9/84 ENGINEER CM	<u>.</u> т	ABLE I	PA(DE 26	OF <u>31</u>
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	ОНМ-СМ
410	"A" STREET	5-3"	1.30	10	1000	13,000
411	'A" STREET		7.30	Land		7,300
412	NORTH WEST OF HOSPITAL		9.20	1		9,200
413	NORTH OF HOSPITAL		5.2			5,200
414	NORTH-EAST OF HOSPITAL		8.7	+		8,700
415	AT PUBLIC WORKS BLDG.	\downarrow	1.1	10		11,000
416	UNDERGROUND FUEL TANK, NORTH	5-3	1.3	1	1000	1,300
	\downarrow	10-6"	1.0		2000	2,000
417	, EAST	5-3"	4.4	414	1000	4,400
-	↓	10-61	1.3		2000	2,600
418	, WEST	5-3	3.85		1000	3,850
1.1	\downarrow \downarrow	10-61	1.6		2000	3,200
119	UNDERGROUND FUEL TANK AT	5-3	4.0		1000	4,000
+20	PUBLIC WORKS BLDG. GAS STATION	5-3"	2.9	4	1000	2,900
					· .	· · · ·
					and	
					62.23	<u> </u>
				19.85 C		
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			1			

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GC	PS GE	NERA	L CATHODIC PRO	TECTI	ON	SERVICE	ES IN	c.	W Sage		Mark Cal
TITL	E: CATH	ODIC	PROTECTION SURVEY	, MAR	INE	CORPS B	ASE,	CAM	P LEJEL	JNE	, N.C.
SOIL	RESIST	IVITY	MEASUREMENTS								
STR	UCTURE	: 5	ITE PLAN	a dagida in Sanan ang							and a second
DATE	=/9	/84		/JM		ABLE	<u> </u>	PA	GE 27	_ (OF 31
TEST NO. '		TEST	LOCATION	AVE	RAG	READIN	IG M	IULTI.	FACT	OR	онм-см
430	BREN	NSTE	R BLVD.	5!	31	3.9		10	100	0	39.000
431	STON	E 51.	AT STABLES		1	1.45		1	131		14.500
432	Howo	MBEE	REWSTER BLVD.			2.3			1.81		23.000
433	Horco	AB BL	D. AT WATER WELL			1.9		\downarrow			19.000
434			& WALLACE CREEK	4		3.5		1			3.500
435	4	e 10 g	E BEAR HEAD			1.8	1	0			18.000
436	LYMAN	# SHI	EADS FERRY RD.			2.2	1	Î			22.000
437		ą	is many preserves			4.0		:			40.000
438	122	ŧ 60	whead creek			2.5					25.000
439		\$ 0B	SERVATION POST #3			5.15		4			51.500
440	\downarrow					2.6			1	1	26,000
441	LYMAN	\$ PU ST	ck creek Arung RD.	51	31	3.1			1000	>	31,000
<u></u>		,		10-01	611	1.9			2000	2	38.000
		- 10		151-	91	1.4			3000	2.	42,000
				21-0	21	1.05			4000	2	42,000
42	SPRIN	G B	ranch	5-3	31	9.75			1000		71,500
			and the second	101-6	5"	2.8	2.65		2000	5	6,000
		1 .	A State of the second	15-0	7"	1.85			3000	5	5,500
	1994		,	21-0	21	1.4	1		4000	5	6,000
43	puck c	REEK	STARLING RD.	5-3	31	1.15	100	2	000	11	5,000
	A gene			10-0	5"	6.75	10		2000	13	5,000
				15-0	711	4.75	10		3000	14	2,500
			,	21-0	,	3.3	10	4	1000	13	2.000
	n a saint ann				+		•		•	1	



חדת	LE: CATHODIC PROTECTION SURVEY	, MARINE	CORPS BA	SE, CAMP	LEJEUN	E, N.C.
<u></u> S01	L RESISTIVITY MEASUREMENTS			No.		
STR	UCTURE: SITE PLAN				a sangene an er Galardari	
DAT	E 11/9/84 ENGINEER CM	JM T	ABLE I	PA	ge <u>28</u>	OF 31
TEST NO.	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	е онм-см
444	PUCK CREEK STABLING BR	5.31	3.95	10	1000	39.500
		10:6	2.15	ŀ	2000	43 000
		15-9	1.6		3000	48.000
		21-0"	1.35		4000	54.000
445	DUCK CREEK STARLING RD.	5-3"	1.4	100	1000	140.000
		10-61	4.65	10	2000	13,000
		15-91	2.5	1	3000	15,000
	4	21-01	1.9		4000	1.640
446	PUCK CREEK STARLING RD.	5-31	1.5	+	1000	15.000
	& FREEMANS CREEK	10-61	3.6	I	2000	1,200
		15-91	1.8	1	3000	5,400
	\downarrow	21-01	5.0		4000	20,000
447	SHEADS FERRY RD. & BEACH RD.	5-3"	2.9	10	1000	29,000
448	BEACH RD.	5-31	5.25		1000	52,500
449	SHEADS FERRY RD. & ACCESS RD.	5-31	8.5		1000	85,000
17		10-6"	35		2000	10,000
		15-91	1.95		3000	58,500
	↓	21-01	1.2		4000	48,000
450	SNEADS FERRY RD &	5-31	1.9	+	1000	19,000
	HOLOVER CREEK	10-6"	7.0	1	2000	14,000
		15-9"	3.7		3000	11,100
	↓	21-01	2.8		4000	11,200
451	SNEADS FERRY RD &	5-31	3.55	10	000	35,500
	AMPHIBIAN RD.	0-61	2.4	10 1	2000	18,000



TITL	E: CATHODIC PROTE	CTION SURVEY	MARIN	EC	ORPS BA	SF.	CAMP	IF	JEUNE		
SOIL	RESISTIVITY MEASUR	EMENTS				,				-, 11.0	•
STRU	ICTURE: SITE	PLAN					an a				in an
DATE	11/9/84 EI	NGINEER CM/	ML	TA	BLE]		PA	GE	29	OF 31	
			10.000		1.	T		T			-
NO.	TEST LOCAT	ION .	AVERA	GE H	READING	s M	ULTI.	'	FACTOR	OHM-	ĊM
451	SNEADS FERRY RD.	ŧ.	151.	91	1.85		0	3	000	55.5	0
	AMPHIBIAN RD.		21-0	211	1.45		1	4	000	58.0	200
452	AMPHIBIAN RD	?	56	3	1.2	1		10	000	12,0	0
			10-01	51	4.3			2	000	86.0	0
			「」	7"	2.7			3	000	81,0	00
	· · · · ·		21-6	211	1.85			4	000	14,0	00
453	SNEADS FERRI	RD.	5-3	3	6.15			10	000	61,5	00
			10-0	;	3.15			2	000	63,0	00
1. A.			15-9		2.1			30	000	63,0	0
			21-0	11	1.6		+	40	000	64,0	00
454			5-3		1.15	10	00	10	00	1,150,0	20
· · ·			10-6	"	3.2	10	0	20	000	640,0	20
<u>е с с к</u> «			15-9	"	2.65	10	0	30	00	195,0	100
455			21-0		1.4	16	,	40	00	296,0	201
456	livere Freedom + F		5-3	+	5.1			100	00	51,00	10
457	TNEADS FERRY RUGE	KENCH CREEK		+·	4.45					44,50	10
458		MARINE RU		+	1.1	-	ingen i l		1444	19,00	10
459	40	Driftend Chech		+	31					27 00	0
460	MARINES RD				3.6				- i	36 00	20
461				T	3.0					30 00	20
462				T	1.1					1.00	0
463				1	2.0					70.00	20
464					a		,			8 00	2



	E CAMODIC PROTECTION SURVE	1, MANI		URPS DA	52,	CAM	PLEJE	JNE, N.C.
STD	RESISTIVITY MEASUREMENTS							L. Marriel Mark
DATE	LI / A /A ENQUEED FLAN	1/14					- 00	
DATE	ENGINEER CM		T/	ABLE	-	PA	GE 20	OF 31
TEST NO.	TEST LOCATION	AVER DEP	AGE TH	READING	ML	JLTI.	FACT	FOR OHM-CM
465	MARINES RD.	51-	3"	3.8	1	0	100	0 38,000
466	\downarrow			1.1		1	1	11,000
467	Sheads FERRY RD			2.9				29,000
468				1.6	- 12	1		16,000
469	FRANGE RD.			4.2				42,000
470	GREY POINT RD.		5	3.9	a series	1		39,000
411				4.85				48,500
472				3.55				35,500
473	\checkmark			2.5				25,000
474	VERONA RD.			1.3				13,000
415				4.6				46,000
416				1.3				13,000
411	4		\square	4.4	-	,		44,000
418	VERONA AT U.S. 17	V	_	4.0				40,000
479	CURTIS RD. AT GUARD HOUSE	5-3		1.6			1000	76,000
	(GEIGER)	10-6	,"	9.3	e de	1. A	2000	186,000
	•	15-9	"	1.1			3000	51,000
180	Curtis RD.	5-3	"	1.1			1000	11,000
481	AT MC EXCHANGE			1,4	-			14,000
-82	AT ELEM. SCHOOL	+	1	3.0	V		4	30,000
120			1		an eg antise			
			\bot					
	Enders in a contrain space in the Martin State							


SOIL	RESISTIVITY MEASUREMENTS	MAINE		6 61	-	
DATE	11/6/84 ENGINEER	<u> </u>	ABLE I	PA	GE 31	OF 31
TEST NO.'	TEST LOCATION	AVERAGE DEPTH	READING	MULTI.	FACTOR	OHM-
490	WEST OF TANK FARM .	5-3	2.5	10	1000	25,4
		10-6	1.9	10	2000	38.0
		15-91	0.1	I.	3000	30,3
	\downarrow	21-01	17.6	1	4000	30,4
491	NORTH OF TANK FARM	5-31	2.2	10	1000	22,0
		10-61	1.1	10	2000	22,0
		15-91	8.5	1	3000	25,5
	\downarrow	21-01	4.5	1	4000	18,00
492	TANK FARM BETWEEN TANK 12#13	5-31	4.8	10	1000	48,0
		10-61	1.6	10	2000	32,00
		15-9"	1.8	10	3000	54,0
	\rightarrow	21-01	6.0		4000	24,0
493	South of TANK FARM	5-3	2.4	10	1000	24,0
		10-6	1.2	10	2000	24,9
		15-9	8.5		3000	25,5
1	\checkmark	21-01	6.5	1	4000	26,00
494	MAIN EXCH. GAS STATION	5-31	1.1.	10	1000	11,00
		0-6	4.3	1.00	2000	8,60
495	and the second	51-31	9.8	1 18	1000	9,80
	· ↓	10-6"	6.1	1	2000	13,40



MD	A MENENDEZ- DO	NNELL & ASS	OCIATES, I	NC.	HOUSTON, TEXAS
GCF	S GENERAL CAT	HODIC PROTEC	TION SERV	ICES INC.	
TITLE	CATHODIC PROTEC	TION SURVEY, I	MARINE COR	PS BASE, C	AMP LEJEUNE, N.C.
CURRE	NT REQUIREMENT TE:	ST			
STRU	CTURE: TANK F	ARM , INDL	ISTRIAL A	REA 2	1
DATE	11/8/84 EN	GINEER N.E.	TABL	E III-A	PAGE 1 OF 2
		POTENTI	IAL MEASURE	MENTS	
REF.	LOCATION	STATIC	CURRENT	APPLIED	REMARKS
		VOLTS	VOLTS	VOLTS	
350	TANK#1	421			DUE TO THE HIGH OUR -
351		446	ander Strand an der Sterne der Sterne Sterne von der Sterne d		BENT DEMAND AND HIGH
352	↓ ↓	346			SOIL RESISTIVITY, ATTEM.
353	TANK #2	437			PTS TO SET UP ATEMPO.
354		507			FARY GROUNBED AND
355	-	491		268 A.L	POWER GOURCE WERE
356	TANK #3	515			NOT SUCCESSFUL,
357		516			THEREFORE NO IMPRESSED
358		477		in Salt	CURRENT MEAGUREMENTS
359	TANK #4	510			WERE TAKEN.
360		378			
361		510			
362	\downarrow	501		1	
363	TANK #5	437			
364		514			
365	4	447			
366	TANK #6	445			
367		458			
368		501	gran and any		
369	\downarrow	452			
370	TANK #7	515			
371		534			
372	+	448		en den son son Redes de son te	
373	TANK #8	518		4.5.91 4	

...



MD	A MENENDEZ- DON	INELL & ASS	OCIATES,	INC.	HOUSTON, TEXAS
GCF	PS GENERAL CATH	IODIC PROTEC	TION SER	VICES INC.	
TITLE	E: CATHODIC PROTECT	ION SURVEY, N	MARINE COF	RPS BASE, CA	MP LEJEUNE, N.C.
CURRE	ENT REQUIREMENT TEST	r		and the second	
STRU	CTURE: TANK F	FARM, IND	ngtrial	AREA	
DATE	11/8/84 ENG	INEER N.E.	TABL	E III-A P	AGE 2 OF 2
		POTENTI	AL MEASURI	EMENTS	
REF. NO.	LOCATION	STATIC	ĊURRENT	APPLIED	REMARKS
		VOLTS	VOLTS	VOLTS	
374	TANK #8	528			
375		477			
376		547			
377	TANK #15	494			
378		488			
379	and the state	47B	1		
380	4	402	:		
381	TANK #14	520	112.8		
382		507	- 542		
333	↓ ↓	508			
384	TANK #13	508			
385	↓ ↓	536			
386	TANK # 12	538			
387		501			
388		536		1	
389	TANK#11	498	1 and and		
390	4	554			
391	TANK #9	481			
392	and a second	494		a contract and	and the second second
393	↓	486		•	Sime
394	TANK#10	402		and the second second	
395	(600,000 GAL.)	418			
396		429		an dia panganan pertampan Pertampinan pertampinan p Pertampinan pertampinan pertamp	
397	\downarrow	409			



CURRE	CATHODIC PROTECTI	ON SURVEY,	MARINE COR	PS BASE, CA	MP LEJEUNE, N.C.
STRU	CTURE: MAIN EXC	HANGE, C	GAS STAT	NON	
DATE	11/3/84 ENGI	NEER G.M. /	J.H. TABL	E III-B P	AGE I OF I
		POTENT	IAL MEASURE	MENTS	
REF.	LOCATION	STATIÇ	CURRENT	APPLIED	REMARKS
NO.		VOLTS	VOLTS	VOLTS	
		0 AMPS	0.4 AMPS	0.6 AMPS	
400	30,000 GAL.	453	-1.20	-1.66	
401	WEST TANK	477	-2.15	-2.43	<i>!</i>) · · · · ·
402		469	-2.32	-2.64	PRAIN POINT
403		475	848	956	
404	Ļ	464	659	748	
405	30,000 GAL.	494	694	786	
406	CENTER TANK	469	731	847	
407		451	-1.02	-1.23	5 t
408	•	477	819	939	
409	10,000 GAL.	497	684	772	
410	EAST TANK	474	711	807	
41		450	805	916	
412	\rightarrow	480	690	784	
413	10,000 GAL.	431	582	(667	· · · · · · · · · · · · · · · · · · ·
414	NORTH TANK	439	582	668	
415		384	583	680	
416		427	619	703	
417	\downarrow	541	709	791	
					er and an arrest sector



TITLE	CATHODIC PROTECTIO	ON SURVEY,	MARINE COF	RPS BASE, CA	MP LEJEUNE, N.C.
CURRE	NT REQUIREMENT TEST				
STRUC	TURE: 10,000 G.	ALLON TA	NK, RI	-LE RANG	e area 17
DATE	11/0/84 ENGI	NEER N.E. / a	4.4. TABL	E III-G	PAGE 1 OF 1
		POTENT	IAL MEASURI	EMENTS	and the same the
REF. NO.	LOCATION	STATIC	CURRENT	APPLIED	REMARKS
		VOLTS	VOLTS	VOLTS	Constant and
		0 AMPS	800 mA	250mA	
420	NORTH SIDE	436	-1.70	944	
421	East SIDE	420	-3.21	-1.46	
422	SOUTH SIDE	413	- 3.14	-1.33	
423	WEST GIDE	424	-3.06	-1.27	
					-
224	TOP OF TANK	335	-1.98	-1.001	
225	AT VENT PIPE	469	- 6.50	-1.98	DRAIN POINT
	1				3
		a i bu na na sita a Ban ata ing kata	n in constants. I se normalista	generative en Claire. Anti-Antonio Claire	



TITLE	CATHODIC PROTECT	ION SURVEY,	MARINE COR	PS BASE, C	AMP LEJEUNE, N.C.
CURRE	NT REQUIREMENT TES	T			
STRU	CTURE : THREE FUEL	TANKS AT G	as station	J, COURT	House bay area 18
DATE	11/0/84 ENG	INEER N.E. /	G.G. TABLE		PAGE 1 OF 1
. And		POTENI	TIAL MEASURE	MENTS	
REF. NO.	LOCATION	STATIC	CURRENT APPLIED		REMARKS
		VOLTS	VOLTS	VOLTS	
		OAMPS	.40 AMPS		
430	NORTH TANK	363	-1.743	$\left(\right)$	CONTINUITY BETWEEN
					THE THREE TANKS
431		477	-2.08	:	WASS ESTABLISHED
					AT THE VENT PIPES
432	*	439	803		(6000 GAL.)
					:
433	CENTER TANK	470	-2.75		
434		396	713		
125		- 077	- 612		
499	¥	211	040	e	,
436	GOUTH TANK	261	- 962	a trape again	
	1				
437		447	-2.70	1	
		-			
1.18				·	
11				feed and the first states	
- 10 Julio		e papers en la Correction			



TITLE	CATHODIC PROTECT	TION SURVEY,	MARINE COR	PS BASE, CAM	P LEJEUNE, N.C.
CURRE	NT REQUIREMENT TES	Т		a second and	and the second second
STRU	CTURE: 10,000 0	FALLON TAN	IK, #2 FUE	L, ONSLOW	BEACH AREA 19
DATE	11/6/84 ENG	BINEER C.M. / J	.H. TABLI	E III-E PA	GE 1 OF 1
		POTENT	IAL MEASURE	MENTS	
REF. NO.	LOCATION	STATIC	CURRENT	APPLIED	REMARKS
		VOLTS	VOLTS	VOLTS	
		0 AMPS	9.8AMPS		
440	WEST SIDE	510	902		
*					
441	North Give	449	729		
442	EAST SIDE	511	963		· · · · · · · · · · · · · · · · · · ·
443	SOUTH SIDE	508	713		
					<u> </u>
		,			
			-		
	Tela				
		- Coloridad			
		a and a state			n and the second
				1	
		11.00			and the second se



S GENERAL CATH	ODIC PROTE	CTION SER	VICES INC.	
: CATHODIC PROTECŢI	ON SURVEY,	MARINE COR	RPS BASE, CA	MP LEJEUNE, N.C.
NT REQUIREMENT TEST				
TURE: 10,000 GAL	LON TANK,	#2 FUEL,	FRENCH C	reek area 20
11/7/84 ENGI	NEER GM. /.	J.H. TABL	E III-F	PAGE _ OF _
	POTENT	TIAL MEASUR	EMENTS	
LOCATION	STATIC	CURRENT	APPLIED	REMARKS
	VOLTS	VOLTS	VOLTS	
	OAMPS	loomA	1	
SOUTH SIDE	483	646		DRAIN POINT
	144		i	
EAST SIVE	400	-1.100		1
NORTH SIDE	459	563	,	
WEST SIDE	473	580		
TOP CENTER	502	047		
TANK LOCATED	GOLTH-5	AGT OF	BLDG. FC	- 202
·				
1				
	and the			
	The second	4		
	ale antinea lider			
				<u></u>
	Sec. Sec.			· · · · · · · · · · · · · · · · · · ·
	PS GENERAL CATHO CATHODIC PROTECTION TREQUIREMENT TEST CTURE: 10,000 GAL 11/7/84 ENGI LOCATION GOUTH SIDE BAST SIDE WEST SIDE TOP CENTER TANK LOCATED	2S GENERAL CATHODIC PROTECTION SURVEY, IT REQUIREMENT TEST CTURE: 10,000 GALLON TANK, 11/7/84 ENGINEER GM./. LOCATION STATIC VOLTS 0 AMPS GOUTH SIDE 463 BAST SIDE 463 NORTH SIDE 459 WEST SIDE 473 TOP CENTER 502 TANK LOCATED SOUTH-E INDE 473 INDE 502 INDE INDE INDE INDE INDE INDE INDE INDE INDE INDE INDE INDE INDE INDE </td <td><math display="block">\frac{25}{3} \text{ GENERAL CATHODIC PROTECTION SURVEY, MARINE CONSTRUCT REQUIREMENT TEST CTURE: 10,000 GALLON TANK, #2 FUEL, 11/7/84 ENGINEER GM./J.H. TABL POTENTIAL MEASURA LOCATION $\frac{11/7/84}{2} \text{ ENGINEER GM./J.H. TABL}$ $\frac{11/7/84}{2} \text{ ENGINEER GM./J.H. TABL}$ $\frac{11/7/84}{2} \text{ ENGINER GM./J.H. TABL}$ $\frac{11/7}{2} \text{ ENGINE GM./J.H. TABL}$ $\frac{11/7}{2} ENGINE GM./$</math></td> <td>PS GENERAL CATHODIC PROTECTION SERVICES INC. : CATHODIC PROTECTION SURVEY, MARINE CORPS BASE, CA NT REQUIREMENT TEST CTURE : 10,000 GALLON TANK, *2 FUBL, FRENCH (2000) 11/7/84 ENGINEER CM./J.H. II/7/84 ENGINEER CM./J.H. LOCATION STATIC CURRENT APPLIED VOLTS VOLTS VOLTS STATIC CURRENT APPLIED VOLTS VOLTS STATIC CURRENT APPLIED VOLTS VOLTS VOLTS VOLTS VOLTS VOLTS STATIC CURRENT APPLIED VOLTS VOLTS VORTH SIDE 4563 VEGET SIDE 5602 TANK LOCATED SOUTH-SAGT OF</td>	$\frac{25}{3} \text{ GENERAL CATHODIC PROTECTION SURVEY, MARINE CONSTRUCT REQUIREMENT TEST CTURE: 10,000 GALLON TANK, #2 FUEL, 11/7/84 ENGINEER GM./J.H. TABL POTENTIAL MEASURA LOCATION \frac{11/7/84}{2} \text{ ENGINEER GM./J.H. TABL} \frac{11/7/84}{2} \text{ ENGINEER GM./J.H. TABL} \frac{11/7/84}{2} \text{ ENGINER GM./J.H. TABL} \frac{11/7}{2} \text{ ENGINE GM./J.H. TABL} \frac{11/7}{2} ENGINE GM./$	PS GENERAL CATHODIC PROTECTION SERVICES INC. : CATHODIC PROTECTION SURVEY, MARINE CORPS BASE, CA NT REQUIREMENT TEST CTURE : 10,000 GALLON TANK, *2 FUBL, FRENCH (2000) 11/7/84 ENGINEER CM./J.H. II/7/84 ENGINEER CM./J.H. LOCATION STATIC CURRENT APPLIED VOLTS VOLTS VOLTS STATIC CURRENT APPLIED VOLTS VOLTS STATIC CURRENT APPLIED VOLTS VOLTS VOLTS VOLTS VOLTS VOLTS STATIC CURRENT APPLIED VOLTS VOLTS VORTH SIDE 4563 VEGET SIDE 5602 TANK LOCATED SOUTH-SAGT OF



TITLE	CATHODIC PROTECT	ION SURVEY,	MARINE COR	PS BASE, C	AMP LEJEUNE, N.C.
CURRE	NT REQUIREMENT TEST	r		and the second	and the state of the second second
STRU	CTURE: 10,000 GALL	ON TANK, G	AS STATION	NAT PUBLI	C WORKS, AREA 21
DATE	11/8/84 ENG	INEER C.M. / J	H. TABL	E III-G	PAGE 1 OF 1
		POTENT	IAL MEASURE		
REF.	LOCATION	STATIC	CURRENT	APPLIED	REMARKS
		VOLTS	VOLTS	VOLTS	1
		OAMPS	235 mA	A Stall	
460	NORTH SIDE	621	-1.07		
		· · · · · · · · · · · · · · · · · · ·			
461	EAST SIDE	565	-1.01		
462	SOUTH SIDE	601	-1.22		PRAIN POINT
163	WEET GIPE	- 513	- 990	:	
407	WEST SITE	247	. 110		
464	TOP CENTER	576	970		
					and the second second
				а ,	中的民族的社会主要
	4				
		1.332		• •	
				and the second	
	· · · · · · · · · · · · · · · · · · ·	1.336.53			
-				· ;	
				A CONTRACTOR	
				1. 18 A.	
		1 Takin Mara			

2.02

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M D A MENENDEZ- DONNELL & ASSOCIATES, INC.

HOUSTON, TEXAS

GCPS GENERAL CATHODIC PROTECTION SERVICES INC.

TITLE: CATHODIC PROTECTION SURVEY, MARINE CORPS BASE, CAMP LEJEUNE, N.C.

CONTINUITY TEST DATA

STRUCTURE: FIRE WATER LINES

DATE 11/7/84 ENGINEER N.E. / G.G. TABLE IV PAGE 1 OF 4

							A REAL PROPERTY AND A REAL	
		STRUCT.	-TO-SOIL	POTENTIAL	(VOLTS)		and and the second second	
TEST NO.	SECTION OF	CLO	DSE	REM	OTE	REF.	REMARKS	
		I-ON	I-OFF	I-ON	I-OFF			
	HADNOT POINT 1	, AREA	4	1.25.278				
500A	FIRE HYPRANT ON	-1.006	542			ATA	1	
	EIVER RO. AT BLOG							
	123		4					
3008	FIRE HYDRANT ON	468	468			AT'B'		
	B'STREET & RIVER	936		426		AT'B'	NO CONTINUITY	
	20.				er - en er Heren er	1		
	OLD HOSPITAL,	AREA	5					
AID	FIRE HYDRANT ON	-1.162	433			AT'A'		
	RIVER RD. AT BLOG.							
	45 '							
DIB	FIRE HYDRANT ON	492	492			AT'B'		
	ENER RD. AT BLOG.	-1.162		492		AT'B'	NO CONTINUITY	
	5		al the state		ten telepes ag			
			Sec. 1					
		Market Start		Constant of				
	an all a statistic ter	the start			provident (free N	1.1		
-	al-gale in the second	generative.	al al al al	Constant P	The State	1. 201. 1	and the second	

NOTES:

SEE PWG. NO. SK-GI48-A FOR TEST PROCEDURE



M D A MENENDEZ- DONNELL & ASSOCIATES, INC.

HOUSTON, TEXAS

GCPS GENERAL CATHODIC PROTECTION SERVICES INC.

TITLE: CATHODIC PROTECTION SURVEY, MARINE CORPS BASE, CAMP LEJEUNE, N.C.

CONTINUITY TEST DATA

STRUCTURE: FIRE WATER LINES

DATE 11/7/84 ENGINEER N.E./ G.G. TABLE IV

	PAGE	2	OF	4
121-123	and the second second	Distance in the		1000

TEST NO. PA 502A FIR 5E BLV 502B FIR 5ET BLV	SECTION OF LINE TESTED REDISE PO E HYDRANT ON TH WILLIAMS D., AT BLOG. OB E HYDRANT ON	CLO I-ON NT, -1.361	I-OFF	REM I-ON 7	IOTE I-OFF	LOCAT.	REMARKS
502A FIR 502A FIR 502B FIR 502B FIR 502B FIR 502B FIR	E HYDRANT ON TH WILLIAMS D., AT BLOG. OB E HYDRANT ON	I-ON NT, -1.361	I-OFF AREA 460	I-ON 7	I-OFF	 ^	
PA 502A FIR 502B FIR 502B FIR 502B FIR 502B FIR 502B FIR	E HYDRANT ON TH WILLIAMS D., AT BLOG. OS E HYDRANT ON	-1.361	AREA 460	7		AT'A'	
502A FIR 502A FIR 502B FIR 502B FIR 502B FIR 502B FIR	E HYDRANT ON TH WILLIAMS D., AT BLOG. OS E HYDRANT ON	-1.361	460			AT'A'	
502B FIRE 502B FIRE 5ETI BLVI	TH WILLIAMS D., AT BLOG. 08 E HYDRANT ON					L	
502B FIRE SETI BLVI	E HYDRANT ON					1.1	and the second second
502B FIRE SETT BLVI	E HYDRANT ON	a set al		1			
SETI BLVI		489	487			AT'B'	4
BLVI	HWILLIAMS	489	:	-1.116		AT'B'	NO CONTINUITY
	D., AT BLOG. 04						
TAI	rawa terr	ACE I	, AREA	. 11			
503A FIR	E HYDRANT ON	-2.44	497			AT'A'	
TAP	AWA BLVD. AT			•		1	
BLE	06.2012						
5038 FIRE	HYDRANT ON	477	477	1		AT'B'	
TAR	AWA BLUD. AT	477		-2.91		AT'B'	NO CONTINUITY
BLC	G. 2072						· · ·
				1.0			
		Ser Barrier	8	;		· · · · · · · · · · · · · · · · · · ·	
		n de state Alte contra					
				1			
NOTES:		Serie Mar to the			the second s		



TITLE	E: CATHODIC PROTECT	ION SURV	EY, MAR	INE CORP	S BASE.	CAMP LEJ	EUNE, N.C.
CONTI	NUITY TEST DATA	a A Acteur		-	and man	Sec. 2	and the second sec
STRU	CTURE: FIRE WA-	ter Li	NES				
DATE	11/3/84 ENG	NEER N.	E./G.G.	TABLE	IV	PAGE	3 OF 4
		STRUCTTO-SOIL POTENTIAL(VOLTS)					1
TEST NO.	SECTION OF LINE TESTED	CLOSE		REMOTE		REF.	REMARKS
		I-ON	I-OFF	I-ON	I-OFF	_ LUCAT.	ALTAKKS
	MONTFORD PO	INT ,	AREA	14	1.211	1. A. 1. 4	
504A	FIRE HYDRANT ON	790	404			AT'A'	
	MONTFORD RD. AT					- ' -	
	BLDG. M-142						
		- internation					and the second se
504B	FIRE HYDRANT ON	470	470			AT'B'	
	MONTFORD RD. AT	920		477		AT'B'	NO CONTINUITY
	BLDG. M-128						
	CAMP GEIGE	ER,	AREA	15		1.	
505A	FIRE HYDRANT ON	760	491			AT 'A'	
, "	X'ST. € 9™ST.					1.34	
5058	FYRE HYDRANT ON	468	468			AT'B'	
۰.	'A'ST. & IO™ST.	870	-	468		AT'B'	NO CONTINUITY
		and the same		· · · 2		e ser ser ser	
1							
11 200							
			<u>ે</u>				
		******				1.8	



עדו נועודלא
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עדו נועודעי
ידוטטודעי
NTINUITY
ALLINUITY
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RECTIFIER DATA	SURVEY DATA
MFGR. $HARCO$ SERIAL NO. $4/07$ DC RATING 40 VOLTS. 20 AMPS. SHUNT RATING: mV. AMPS. TAP COURSE A A SETTINGS FINE 3 3 DC OUTPUT $4V$ $4V$ BOWL CURRENT $1A$ $.75A$ RISER CURRENT $.4A$ $.24.$	POTENTIAL PROFILE WET AREA AT SURVEY 75% FULL TANK BOTTOM $1.49V$ $+15$ $62V$ $+30$ $1.46V$ $+3$ $1.52V$ $+18$ $1.60V$ $+33$ $500RFACE$ $+3$ $1.52V$ $+18$ $1.60V$ $+33$ $500RFACE$ $+3$ $1.52V$ $+18$ $1.60V$ $+33$ $500RFACE$ $+6$ $1.57V$ $+21$ $1.57V$ $+36$ $+9$ $1.61V$ $+24$ $7.53V$ $+39$ $+12$ $1.62V$ $+27$ $1.48V$ 0 OFF POTENTIAL $1.04V$ $1.R. DROP$ $250MV$ ANODE STRING CURRENT DRAINS $(going counterclockwise from ladder)$ 0 UTER RING $1.NNER RING$ 1 $0.00A$ 1 $0.00A$ $2.005A$ $1.00A$ 2 $0.065A$ $1.200A$ $2.015A$
COMMENTS: ROOF MAN-WAY IS PETACHED (RUSTED OFF) CONDULET COVERS ON BALCONY ARE MISSING HARDWARE O.K. INTERIOR COATING LOOKED GOOD ANODES ~ G TO 8 YEARS LIFE	3 0.050A 4 0.055A 5 0.055A 5 0.055A 5 0.055A 6 0.055A 7 0.060A 8 0.060A 9 0.060A 10 0.060A $RISER . 18A 9 0.060A RISER . 18A RISER . 18A $





RECTIFIER DATA	SURVEY DATA
MFGR. <u>HARCO</u> SERIAL NO. <u>4106</u> DC RATING <u>18</u> VOLTS. <u>16</u> AMPS. SHUNT RATING: mV. AMPS. <u>AS FOUND</u> <u>AS LEFT</u> TAP COURSE <u>B</u> <u>A</u> SETTINGS FINE 1 1 DC OUTPUT <u>6.2 V</u> <u>2.3 V</u> BOWL CURRENT 1.3 A .65 A RISER CURRENT .30 A .015 A	POTENTIAL PROFILE WET AREA AT SURVEY 50% FULL TANKBOTTOM $1.26V.$ $+151.25V.$ $+30$ $+31.24V.$ $+181.22V.$ $+33$ $+61.28V.$ $+211.20V.$ $+36$ $+91.25V.$ $+241.21V.$ $+39$ $+121.26V.$ $+271.19V.5URFACE$ OFF POTENTIAL $1.03V.$ I.R. DROPOFF POTENTIAL $1.03V.$ I.R. DROPOUTER RINGINNER RING $1.020A.$ $1.05A.$
COMMENTS: ANORES & TO 8 YEARS LIFE HARDWARE O.K INTERIOR COATING LOOKED GOOD	$2 \cdot 020A. 2 \cdot 05A.$ $3 \cdot 0/5A. 3 \cdot 08A.$ $4 \cdot 0/5A. 4 \cdot 08A.$ $5 \cdot 0/5A. 5 \cdot 08A.$ $6 \cdot 0/5A. 5 \cdot 08A.$ $6 \cdot 0/5A.$ $7 \cdot 0/5A.$ $8 \cdot 020A.$ $8 \cdot 020A.$ $9 \cdot 020A.$ $10 \cdot 020A.$ $BATE OF SURVEY - NOV. 7, 1984$





RECTIFIER DATA	SURVEY DATA
MFGR. <u>HARCO</u> SERIAL NO. <u>4103</u> DC RATING <u>18</u> VOLTS. <u>10</u> AMPS. SHUNT RATING: mV. _AMPS. <u>AS FOUND</u> <u>AS LEFT</u> TAP COURSE <u>A</u> SETTINGS FINE <u>1</u> DC OUTPUT <u>3.964</u> <u>3.964</u> BOWL CURRENT .60A .60A RISER CURRENT .12A .12A	POTENTIAL PROFILE WET AREA AT SURVEY FULL TANK. BOTTOM $1.36V$ $+15$ $1.61V$ $+30$ $1.53V$ $+3$ $1.50V$ $+15$ $1.61V$ $+30$ $1.53V$ $+3$ $1.50V$ $+18$ $1.60V$ $+33$ $1.54V$ $+6$ $1.55V$ $+21$ $1.57V$ $+36$ $1.52V$ $+9$ $1.59V$ $+24$ $1.54V$ $+39$ $1.48V$ $+12$ $1.61V$ $+27$ $1.51V$ $60PFACE$ OFF POTENTIAL $1.07V$ $1.R. DROP$ $250MV$ ANODE STRING CURRENT DRAINS (going counterclockwise from ladder) OUTER RING INNER RING $1.008A$ $2.010A$
COMMENTS: INNER ARRAY MISSING ONE STRING ANODES - 5 TOTYRS LIFE HARDWARE O.K. INTERIOR COATING LOOKED GOOD	$3 \cdot 046A \cdot 3 \cdot M1551NG$ $4 \cdot 050A \cdot 4 \cdot 011A \cdot 5 \cdot 048A \cdot 5 \cdot 009A \cdot 6 \cdot 045A \cdot 5 \cdot 009A \cdot 6 \cdot 046A \cdot 8 \cdot 050A \cdot RISER \cdot 14A \cdot 9 \cdot 045A \cdot 10 \cdot 045A \cdot 104 \cdot $





MFGR.GOOP-ALLSERIAL NO. $BOC 2899$ POTENTIAL PROFILE WET AREA AT SURVEY $50\% F00L 7$ DC RATING 40 VOLTS. 20 AMPS.SHUNT (Bowl) $.0014$ mV. $.70$ AMPS.RATING (Riser) $.0022$ mV. $.22$ AMPS.AS FOUNDAS LEFTTAP SETTINGSCOURSE A A A DC OUTPUT $4V$. $4V$ BOWL CURRENT $.45A$. $.45A$ RISER CURRENT $.20A$. $.20A$. $.20A$.COMMENTS: ARDDES- 500 T YRS LIFE $ANDDES 5 T0$ T YRS LIFE $AREPWARE O.K$ $5.060A$ $INTERIOR CONTRY200KEP GOOP.ANDDES 5 T0 T YRS LIFEAREPWARE O.K6.055AINTERIOR CONTRY200KEP GOOP.RISER_0AARISER_20AARISER_0AARISER_20AA$	SURVEY DATA	RECTIFIER DATA
COMMENTS: 3 .060.4. 3 .010.4. ANODES- 5 TO T YR5 LIFE 4 .060.4. 4 .025.4. HARDWARE 0.K. 5 .060.4. 5	POTENTIAL PROFILE WET AREA AT SURVEY $\underline{50\%}$ FULL FULL $TANK$ BOTTOM $\underline{/.32V.}$ $\pm 15 \underline{/.56V.}$ $\pm 30 \underline{)}$ $\pm 3 \underline{/.43V.}$ $\pm 15 \underline{/.56V.}$ $\pm 30 \underline{)}$ $\pm 3 \underline{/.43V.}$ $\pm 18 \underline{/.53V.}$ $\pm 33 \underline{)}$ $\pm 6 \underline{/.53V.}$ $\pm 21 \underline{/.48V.}$ $\pm 36 \underline{)}$ $\pm 9 \underline{/.57V.}$ $\pm 24 \underline{/.44V.}$ $\pm 39 \underline{)}$ $\pm 12 \underline{/.58V.}$ $\pm 27 \underline{)}$ OFF POTENTIAL $\underline{/.07V.}$ I.R. DROP 200 MV.ANODE STRING CURRENT DRAINS (going counterclockwise from ladder)OUTER RING $1 \underline{.025A.}$ $1 \underline{.025A.}$ $2 \underline{.050A.}$ $2 \underline{.012A.}$	MFGR. <u>GOOP-ALL</u> SERIAL NO. <u>BOC 2889</u> DC RATING <u>40</u> VOLTS. <u>20</u> AMPS. SHUNT (Bowl) <u>0014</u> mV. <u>70</u> AMPS. RATING (Riser) <u>0022</u> mV. <u>22</u> AMPS. MFGR. <u>AS FOUND</u> <u>AS LEFT</u> TAP <u>COURSE</u> <u>A</u> SETTINGS FINE <u>3</u> DC OUTPUT <u>4V</u> <u>4V</u> BOWL CURRENT <u>454</u> <u>454</u> RISER CURRENT <u>204</u> <u>204</u>
	3 .060A. 3 .010A. 4 .060A. 4 .025A 5 .060A. 5 6 .055A. 7 .055A. 8 .060A. RISER .20A. 9 10	COMMENTS: ANODES- 5 TO T YRS LIFE HARDWARE O.K INTERIOR COATING LOOKED GOOD.





RECTIFIER DATA	SURVEY DATA
MFGR. <u>HARCO</u> SERIAL NO. <u>5201</u> DC RATING <u>36</u> VOLTS. <u>16</u> AMPS. SHUNT RATING: mV. AMPS. <u>AS FOUND</u> <u>AS LEFT</u> TAP COURSE <u>A</u> SETTINGS FINE <u>3</u> DC OUTPUT <u>5.4 V</u> <u>5.4 V</u> BOWL CURRENT <u>14</u> <u>14</u> RISER CURRENT <u>20A</u> <u>20A</u>	POTENTIAL PROFILE WET AREA AT SURVEY 60% FULL TANK. BOTTOM $(.45V.)$ $+15/.54V.$ $+30$ $+3$ $(.49V.)$ $+18/.52V.$ $+33$ $+6$ $(.50V.)$ $+21/.49V.$ $+36$ $+9$ $(.54V.)$ $+21/.49V.$ $+36$ $+9$ $(.54V.)$ $+24/.48V.$ $+39$ $+12$ $(.54V.)$ $+27$ $=$ OFF POTENTIAL $1.07V.$ $I.R. DROP$ $250 MV.$ ANODE STRING CURRENT DRAINS $(going counterclockwise from ladder)$ $OUTER RING$ $INNER RING$ $1 - 070A.$ $1 - 035A.$ $2 - 025A.$ $2 - 025A.$
COMMENTS: ANODES ~ 5 TO 7 YRS LIFE HARDWARE O.K. INTERIOR COATING LOOKED GOOD.	$3 - \frac{120 A}{100 A} = 3 - \frac{045A}{4}$ $4 - \frac{100A}{4} = 4$ $5 - \frac{095A}{5} = 5$ $6 - \frac{100A}{4} = 5$ $6 - \frac{100A}{4} = 5$ $RISER - \frac{0.17A}{4}$ $9 - \frac{10}{10} = 5$ $RISER - \frac{0.17A}{4} = 5$




RECTIFIER DATA	SURVEY DATA		
MFGR. GOOD-ALL SERIAL NO. BOC 2834 DC RATING 40 VOLTS. 20 AMPS. SHUNT (Bowl).00/5 mV. 75 AMPS. RATING (Riser).00/8 mV. 78 AMPS. RATING (Riser).00/8 mV. 78 AMPS. RATING (Riser).00/8 mV. 78 AMPS. TAP COURSE A A SETTINGS FINE 3 3 DC OUTPUT 4.62 V. 4.62 V BOWL CURRENT .58.4 .58.4. RISER CURRENT .18.4 .18.4 COMMENTS: .18.4 .18.4 AMODES ~ S TO T YES LIFE	POTENTIAL PROFILE SO % FULL TANK BOTTOM $1.38V$, +15 $1.66V$, +30 +3 +3 $1.54V$, +18 $1.61V$, +33 +3 +6 $1.64V$, +21 $1.58V$, +36 +3 +9 $1.67V$, +24 +39 +12 $1.68V$, +27		
	DATE OF SURVEY- NOV. 8, 1984		





RECTIFIER DATA	SURVEY DATA		
ALECTIFIER DATA MFGR. \underline{HARCO} SERIAL NO. <u>5630</u> DC RATING 18 VOLTS. 16 AMPS. SHUNT RATING: mV. AMPS. SHUNT RATING: mV. AMPS. MFGR. AS LEFT TAP COURSE A SETTINGS FINE 3 DC OUTPUT $3V$ $3V$ BOWL CURRENT $.4A$ $.4A$ RISER CURRENT $.06A$ $.06A$ COMMENTS: PISEE HANDHOLE COVER MISSING BAE & BOLT. ANOPES 5 TO T YES LIFE HARDWARE O.K. INTERIOR COATING LOOKED GOOD	SURVEY DATA POTENTIAL PROFILE WET AREA AT SURVEY $74NK$ BOTTOM $1.43V$ $+15$ $1.49V$ $+3$ $1.45V$ $+3$ $1.45V$ $+3$ $1.45V$ $+3$ $1.45V$ $+3$ $1.45V$ $+3$ $1.45V$ $+3$ $1.49V$ $+3$ $1.49V$ $+3$ $1.49V$ $+3$ $1.49V$ $+3$ $1.49V$ $+24$ $1.52V$ $+39$ $1.46V$ $+24$ $1.52V$ $+39$ $1.46V$ $+27$ $1.53V$ OFF POTENTIAL $1.02V$ I.R. DROP $300MV$ ANODE STRING CURRENT DRAINS (going counterclockwise from ladder) OUTER RING INNER RING 1 $0.25A$ 2 $040A$ 2 $0.40A$ 3 $0.20A$ 4 $0.20A$ 5 $-0.60A$		
	9 10 PATE OF SURVEY- NOV. 8, 1984		







RECTIFIER DATA	SURVEY DATA
MFGR. HARCO SERIAL NO. 12210 DC RATING 18 VOLTS. 10 AMPS. SHUNT RATING: mV. AMPS. TAP COURSE A B SETTINGS FINE 4 3 DC OUTPUT 3.63V 6.72V BOWL CURRENT .35A. 1.0 A RISER CURRENT .050A .60A	POTENTIAL PROFILE WET AREA AT SURVEY $TANK$ $FULL$ BOTTOM $1.00V.$ $+15$ $1.07V.$ $+30$ $+3$ $1.04V.$ $+15$ $1.07V.$ $+33$ $+6$ $1.08V.$ $+21$ $1.07V.$ $+36$ $+9$ $1.0V.$ $+24$ -436 $+9$ $1.0V.$ $+24$ -436 $+9$ $1.0V.$ $+24$ -439 $+12$ $1.0V.$ $+24$ -439 $+12$ $1.07V.$ $+26$ $+39$ $+12$ $1.07V.$ $+27$ $-390V.$ OFF POTENTIAL $-950V$ $1.R. DROP -50MV.$ ANODE STRING CURRENT DRAINS (going counterclockwise from ladder) $0UTER RING$ 1 $2 - 10A.$ 2 -3 $-10A.$ $3 - 10A.$ 3
BE O.K. ANODES 2 TO 3 YRS LIFE HARDWARE O.K. INTERIOR COATING LOOKED GOOD.	5 <u>. 104.</u> 5 6 7 8 9 10 PATE OF GUEVEY - NOV. 13, 1984



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RECTIFIER DATA	SURVEY DATA
MFGR. <u>HARCO</u> SERIAL NO. <u>7236</u> D C RATING <u>40</u> VOLTS. <u>12</u> AMPS. SHUNT RATING: mV. AMPS. X MPS. AMPS. X AMPS. AMPS. Y AMPS. AMPS. DC OUTPUT 2.441/. 8.81/. BOWL CURRENT .4554. 3A. RISER CURRENT .10A. .18A.	POTENTIAL PROFILE WET AREA AT SURVEY70 % FULL TANK.BOTTOM $1.20V.$ $+15$ $+30$ $+3$ $1.20V.$ $+15$ $+30$ $+3$ $1.20V.$ $+15$ $+30$ $+3$ $1.20V.$ $+18$ $+33$ $+6$ $1.98V.$ $+21$ $+36$ $+9$ $1.43V.$ $+24$ $+39$ $+12$ $1.65V.$ $+27$ $5URFACE$ OFF POTENTIAL $.64V.$ $1.R. DROP$ OFF POTENTIAL $.64V.$ $1.R. DROP$ $300 MV.$ ANODE STRING CURRENT DRAINS (going counterclockwise from ladder) $0UTER RING$ 1 1 $.50A.$ 1 $$ 2 $$ 2 $$
<u>COMMENTS:</u> ANORES ~ 5 TO 7 YRS LIFE HARRWARE O.K. INTERIOR COATING LOOKER GOOD.	3 .50A. 3 4 .50A. 4' 5 .55A. 5 6. 7 7 8 RISER /.75A 9
	PATE OF SURVEY NOV. 12, 1984





MFGR. $\underline{4202^{\mu}.42L}$ SERIAL NO. $\underline{\beta_{1/2/15}}$ POTENTIAL PROFILE WET AREA AT SURVEY $\underline{75\%}$ FULL TANK.DC RATINGAMPS.BOTTOM $\underline{1.15\nu}$ ± 15 $\underline{1.51\nu}$ ± 30 SHUNT RATING:MV.AMPS. ± 3 $\underline{1.51\nu}$ ± 30 $\underline{1.32\nu}$ ± 30 SHUNT RATING:MV.AMPS. ± 3 $\underline{1.51\nu}$ ± 30 $\underline{1.32\nu}$ ± 30 SHUNT RATING:MV.AMPS. ± 3 $\underline{1.51\nu}$ ± 30 $\underline{1.32\nu}$ ± 30 TAPCOURSE \underline{A} \underline{A} \underline{A} $\underline{1.22\nu}$ ± 24 ± 39 $\underline{1.22\nu}$ ± 33 DC OUTPUT $\underline{2.06k}$ $\underline{9.02\nu}$ $\underline{9.02\nu}$ $\underline{1.22\nu}$ $\underline{1.8.DROP}$ $\underline{300M\nu}$ BOWL CURRENT $\underline{.24.4}$ $\underline{4.984.}$ $\underline{1.22\nu}$ $\underline{1.8.DROP}$ $\underline{300M\nu}$ RISER CURRENT $\underline{.13.4.}$ $\underline{1.72.4}$ $\underline{1.92}$ $\underline{1.92}$ $\underline{1.92}$ COMMENTS: $\underline{.13.4.}$ $\underline{1.72.4}$ $\underline{1.22\nu}$ $\underline{1.22\nu}$ $\underline{1.22\nu}$ COMMENTS: $\underline{.13.4.}$ $\underline{1.72.4}$ $\underline{1.22\nu}$ $\underline{3.45.4.}$ $\underline{3.22\nu}$ COMMENTS: $\underline{.13.4.}$ $\underline{1.72.4}$ $\underline{1.425.4.}$ $\underline{4.22}$ ANODE STRING CURRENT DRING $\underline{1.425.4.}$ $\underline{4.45.4.}$ $\underline{4.45.4.}$ ANODES -5 TO 7 YES LIFE. $\underline{1.62\nu}$ $\underline{1.452.4.}$ $\underline{4.452.4.}$ $\underline{4.452.4.}$ ANODES -5 TO 7 YES LIFE. $\underline{1.62\nu}$ $\underline{1.452.4.}$ $\underline{4.452.4.}$ <td< th=""><th>RECTIFIER DATA</th><th colspan="2">SURVEY DATA</th></td<>	RECTIFIER DATA	SURVEY DATA	
COMMENTS: 3 .45A. 3	MFGR. GODP-ALL SERIAL NO. BIC 1215 DC RATING GO VOLTS. 28 AMPS. SHUNT RATING: mV AMPS. MFGR. AS FOUND AS LEFT TAP COURSE A A SETTINGS FINE / 3 DC OUTPUT 2.06V. 8.02 V. BOWL CURRENT .24A. 4.98A. RISER CURRENT ./3A. 1.72A	POTENTIAL PROFILE TOP FULL TANK. BOTTOM $1.5V$ $+15$ $5.5V$ $+30$ $+3$ $1.08V$ $+15$ $5.5V$ $+30$ $+3$ $1.08V$ $+18$ $$	
DATE OF SURVEY NOV. 12, 1984	COMMENTS: CONDULET COVER MISSING ON BALCONY. EXTERIOR OF RISER NEEDS PAINTING. ANODES - 5 TO T YRS LIFE. HARDWARE O.K. INTERIOR COATING LOOKED GOOD.	3 <u>.45A</u> 4 <u>.45A</u> 5 <u>.45A</u> 6 7 8 9 10 <i>PATE OF SURVEY NOV. 12, 1984</i>	



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RECTIFIER DATA	SURVEY DATA	
MFGR. <u>GODP.ALL</u> SERIAL NO. <u>BOC 2835</u> DC RATING <u>40</u> VOLTS. <u>20</u> AMPS. SHUNT (Bowl).0072 mV. <u>3.6</u> AMPS. RATING (Riser).0/3 mV <u>1.3</u> AMPS. AS FOUND AS LEFT TAP COURSE <u>B</u> <u>4</u> SETTINGS FINE <u>1</u> <u>3</u> DC OUTPUT <u>10.1 V.</u> <u>4.09 V.</u> BOWL CURRENT <u>3.6 A</u> BOWL CURRENT <u>1.34</u> MASP NEST INSIDE TANK ON SPIPER RODS ANOPES - 5 YRS LIFE HARPWARE O.K. INTERIOR COATING LOOKEP GOOP	POTENTIAL PROFILE FULL $TANK$ BOTTOM $h24V$ $+15$ $h3V$ $+30$ $+3$ $h24V$ $+15$ $h3V$ $+30$ $+3$ $h24V$ $+15$ $h3V$ $+30$ $+3$ $h24V$ $+15$ $h3V$ $+33$ $+6$ $h27V$ $+21$ $+36$ $+9$ $h27V$ $+21$ $+36$ $+9$ $h27V$ $+24$ $+39$ $+12$ $h3V$ $+27$ $-160MV$ OFF POTENTIAL $h/2V$ hR . DROP $100MV$ ANODE STRING CURRENT DRAINS (going counterclockwise from ladder) $0UTER RING$ 1 2 $h24$ 2 -12 -124 2 3 $h4A$ 3 -12 -124 2 -124 -126 3 $h4A$ 1 -126 -126 -126 -126 -126 4 $h17A$ 1 -126 -126 -126 -126 -126 -126 -126 -126	ANSTI
	RISER NO REAPING 9 10 DATE OF SURVEY. NOV. 11, 1984	M G ELE () or R. Scale M

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RECTIFIER DATA	SURVEY DATA
MFGR. <u>HARCO</u> SERIAL NO. <u>4109</u> DC RATING <u>18</u> VOLTS. <u>10</u> AMPS. SHUNT RATING: <u>mV</u> AMPS. <u>AS FOUND</u> <u>AS LEFT</u> <u>TAP</u> COURSE <u>A</u> <u>A</u> <u>SETTINGS</u> <u>FINE</u> <u>1</u> <u>1</u> DC OUTPUT <u>4.8 V.</u> <u>4.8 V.</u> BOWL CURRENT <u>75.4</u> . RISER CURRENT <u>30.A</u> <u>30.A</u> .	POTENTIAL PROFILE WET AREA AT SURVEY 75% FULL TANK. BOTTOM $1.41V$. $+15$ $1.85V$. $+30$ $+3$ $1.96V$. $+15$ $1.36V$. $+33$ $+3$ $1.96V$. $+18$ $1.36V$. $+33$ $+6$ $1.39V$. $+18$ $1.36V$. $+33$ $+6$ $1.99V$. $+21$ $+36$ $+9$ $1.94V$. $+24$ $+39$ $+12$ $1.94V$. $+27$ OFF POTENTIAL $1.79V$. $1.R. DROP$ $200MV$. ANODE STRING CURRENT DRAINS (going counterclockwise from ladder) $0UTER RING$ $INNER RING$ 1 $1.5A$. 1 1 2
<u>COMMENTS:</u> ALL ANORE STRING TIED TO INLET PIPE SHOVEL ON BOTTOM ANORES ~ 5 YRS LIFE HARDWARE O.K. INTERIOR COATING LOOKED GOOD.	3 .15A. 3 4 .15A. 4 5 .15A 5 6
	PATE OF SURVEY NOV. 12, 1984



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RECTIFIER DATA	S	URVEY	DATA	
MFGR. <u>210</u> SERIAL NO. <u>760043</u>	POTENTIAL PRO WET AREA AT	FILE SURVEY <u>SEE</u>	COMMENTS.	
VOLIS. 12 AMPS.	воттом	+ 15	+ 30	12.7
SHUNT RATING:mVAMPS.	+3	+ 18	+ 33	
	+6	+ 21	+ 36	
	+9	+24	+ 39	
AS FOUND AS LEFT	+12	+27		
TAPCOURSE//SETTINGSFINE44	OFF POTENTIAL	1	.R. DROP	
DC OUTPUT <u>81</u> <u>81</u>				
BOWL CURRENT <u>1.08 A</u> <u>1.08A</u>	ANODE STRING (going counterclose	CURRENT DRA ckwise from lac	INS Ider)	
RISER CURRENT <u>·6A</u> <u>·6A</u>	OUTER RING	INNE	ER RING	
	1 .354.	Ι_	<u></u> 29941	
	2 . 35A.	2_		
COMMENTS	3 . 354.	3_	<u> </u>	
MANWAY RUSTED CLOSED & COULD NOT	4 . 35.4.	<u> </u>		
BE OPENED.	5	5 _		
ACCESS HOLE - TOO GLOSE TO INSULATOR.	6			
ALL WIRING APPEARED O.K., HOWEVER HANDHOLES NEED REPLACEMENT.	7			I
EXTERIOR PAINT PEELING BADLY	8	RISE	R	
INTERIOR LIGHTING SYSTEM NON-FUNCTIONAL	9			
	10	i i		
	PATE OF BURN	EY NOV. 11, 1	1984	
entere estado en estado en estado				-







RECTIFIER DATA	SURVEY DATA
MFGR. HARCO SERIAL NO. 7238	POTENTIAL PROFILE WET AREA AT SURVEY SEE COMMENTS
DC RATING 20 VOLTS. 24 AMPS.	BOTTOM +15 +30
SHUNT RATING:MVAMPS.	+3 +18 +33
	+6 +21 +36
	+9 +24 +39
AS FOUND AS LEFT	+12 +27
TAPCOURSEBSETTINGSFINE22	OFF POTENTIAL I.R. DROP
BOWL CURRENT	ANODE STRING CURRENT DRAINS (going counterclockwise from ladder)
RISER CURRENT .35A.	OUTER RING INNER RING
	1 .075A. 1 .015A.
	2 .060A. 2 .015A.
COMMENTS.	3 .058A. 3 .015A.
ROOF LADDER CAN NOT BE'MOVED &	4 .065A 4
OBSTRUCTS ACCESS TO MANWAY.	5 .0654. 5
ALL OBSTRUCTION LIGHTS ARE MISSING	6 .0654.
3/4" CONDULET ON TOP OF TANK IS CRACKED	7 <u>.070 A</u> .
AND MISSING ITS COVER.	8 .0724. RISER .2504
HARPWARE O.K.	9 <u>.070A.</u>
INTERIOR COATING LOOKED GOOD.	10 .0724.
	PATE OF SURVEY NOV. 8, 1984.

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APPENDIX C

SOIL AND WATER ANALYSIS



LOCATION OF SAMPLES

SOIL SAMPLES

- "S-11" Industrial Area 2, from top of tank berm at Fuel Farm.
- "S-12" Industrial Area 2, from vicinity of piping at North end of Fuel Farm.
- "S-13" Hadnot Point 2, Area 3, from pipeline construction trench at "I" Street.
- "S-14" French Creek Area 20, at Reasoner Street.
- "S-15" Montford Point Area 14, from ongoing construction excavation at Montford Road.
- "S-16" Old Naval Hospital Area 5, near Building No. 16.
- <u>"S-17"</u> Berkeley Manor Area 8, from ditch at Stone Street near Marine Corps Exchange # 2.
- "S-18" Courthouse Bay Area 18, at Sneads Ferry Road.
- "S-19" Onslow Beach Area 19, near intersection of Sneads Ferry Road and Access Road.

WATER SAMPLES

- "W-12" Camp Geiger Area 15, from Tank No. S-TC-606.
- "W-13" Midway Park Area 9, from Tank No. S-MP-4004.
- "W-14" Industrial Area 2, from Tank No. S-1000.
- "W-15" Rifle Range Area 17, from Tank No. S-RR-44.
- "W-16" Onslow Beach Area 19, from Tank No. S-BA-108.
- "W-17" Courthouse Bay Area 18, from Tank No. S-BB-25.







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SGS Control Services Inc.

1201 W. 8th Street P.O. Box 550 Deer Park, Texas 77536 Tel: (713) 479-7170 TWX: 910 881 1681 TLX: 795065 SUPERCO DERK December 15, 1984

MENENDEZ-DONNELL & ASSOCIATES 11999 Katy Freeway, #355 Houston, TX 77079 ATTN: Joe Meszaros

Analytical Report No. #97414-2

 LAB REFERENCE NO.:
 L/3445/84
 SAMPLE DESCRIPTION: Soil / Water

 SAMPLE MARKED:
 SUBMITTED SAMPLES AS MARKED BELOW / RECEIVED 12-4-84

 SUBMITTED BY:
 Menendez-Donnell & Associates

RESULTS OF ANALYSIS

Based upon samples submitted to us, tested in our laboratory, reported to you as follows: "WATER SAMPLES"

[Standard Methods 15th Edition]

Method	Tests	"W-12"	<u>"W-13"</u>	"W-14"
423	Total Dissolved Solids, mg/L	397	127	108
209C	pН	8.6	8.1	8.3
426B	Sulfate, mg/L	21.8	10.3	14.8
407C	Chlorides, mg/L	82	13	11
205	Conductivity, µmhos/cm	738	187,	170

	<u>"W-15"</u>	"W-16"	"W-17"
Total Dissolved Solids, mg/L	226	210	202
pH	8.1	7.9	8.4
Sulfate, mg/L	11.5	11.5	7.4
Chlorides, mg/L	19	18	15
Conductivity, µmhos/cm	371	355	360

continued · · · · · · ·

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SGS Control Services Inc.

1201 W. 8th Street P.O. Box 550 Deer Park, Texas 77536 Tel: (713) 479-7170 TWX: 910 881 1681 TLX: 795065 SUPERCO DERK December 15, 1984

MENENDEZ-DONNELL & ASSOCIATES 11999 Katy Freeway, #355 77079 Houston, TX ATTN: Joe Meszaros

Analytical Report No. #97414-2

LAB REFERENCE NO .: L/3445/84

SAMPLE DESCRIPTION: Soil / Water

SUBMITTED SAMPLES AS MARKED BELOW / RECEIVED 12-4-84 SAMPLE MARKED:

SUBMITTED BY: Menendez-Donnell & Associates

RESULTS OF ANALYSIS

Based upon samples, submitted to us, tested in our laboratory, reported to you as follows:

SAMPLES" "SOIL

Tests	<u>"S-11"</u>	<u>"S-12"</u>	<u>"S-13"</u>	"S-14"
pH	7.4			
Sulfate, Wt. % Chlorides, Wt. % Conductivity, µmhos/cm Sodium, ppm Phosphate, Wt.%	7.4 0.001 <0.001 47 19.5 0.013	6.6 0.001 <0.001 205 16.6 0.009	8.6 0.002 <0.001 130 25.3 0.039	7.8 0.002 <0.001 254 22.2 0.005
Carbonate, Wt. %	0.76	0.39	7.02	2.50
<u>"S-15"</u>	<u>"S-16"</u>	<u>"S-17"</u>	<u>"S-18"</u>	<u>"S-19"</u>
9.3 0.002 <0.001 os/cm 224 177 0.345 19.89	6.1 0.001 <0.001 59 19.1 0.049 5.67	7.3 <0.001 <0.001 111 18.7 0.015 7.00	9.5 0.002 <0.001 371 106 0.056 3.80	6.3 0.001 <0.001 87 22.2 0.006 2.32
	Sulfate, Wt. % Chlorides, Wt. % Conductivity, µmhos/cm Sodium, ppm Phosphate, Wt.% Carbonate, Wt. % $\frac{"S-15"}{9.3} \\ 0.002 \\ < 0.001 \\ 0.002 \\ < 0.001 \\ 177 \\ 0.345 \\ 19.89 \\ 19.89$	Sulfate, Wt. % 0.001 Chlorides, Wt. % <0.001 Conductivity, µmhos/cm 47 Sodium, ppm 19.5 Phosphate, Wt.% 0.013 Carbonate, Wt. % 0.76 $\frac{"S-15"}{9.3} \frac{"S-16"}{6.1} \\0.002 0.001 <0.001 \\<0.001 <0.001 \\<0.001 <0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\<0.001 \\\\0.049 \\\\19.89 \\\\0.67 \\$	Sulfate, Wt. % 0.001 0.001 Chlorides, Wt. % <0.001	Sulfate, Wt. % 0.001 0.001 0.002 Chlorides, Wt. % <0.001

Dilution Ratio 1:10

Dilution Ratio 1:1

SGS CONTROL SERVICES INC.

Luch L. Mayo

Hugh L. Mayo,

Laboratory Manager

HLM/bi

Member of the SGS Group (Société Générale de Surveillance)



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APPENDIX D

DESIGN CALCULATIONS



I. POL SYSTEM-INDUSTRIAL AREA

A. Fuel Farm

- The 15 underground tanks at the fuel farm have an exposed surface area of 18376 square feet. Based on a current density of 0.00148 amperes per square foot as calculated for Tank Farm A at Cherry Point Station. Total Current requirement will be 27.2 amperes.
- 2. A rectifier and distributed groundbed are recommended for proper current distribution.
- 3. Weight of anode materials:

Fully treated graphite anodes with calcined fluid petroleum coke backfill are recommended having a deterioration rate of 1-1b per ampere year and a 75% utilization factor.

Design life = 20 years

Weight = 20 years x l-lb/amp-yr x 27.2 amperes = 544 lbs of anode materials

4. Number of Anodes required for 20 years life:

- a. Use fully treated graphite anodes 3-inches diameter x 60 inces long fitted with epoxy and heat shrink cap.
- b. Quantity = 540 lbs x l anode/27-lbs x l/.75 = 27 anodes

.75 is the utilization factor, meaning when the anode is 75% consumed it will require replacement.

Use 30 anodes.

5. Groundbed design

a. Resistance of groundbed to earth: $R = \frac{.00521 f}{NL} \begin{bmatrix} \ln \frac{8L}{D} - 1 + 2 \frac{L}{S} \ln .656(N) \end{bmatrix}$

L = Length of anode and coke column = 7' D = Diameter in ft. = 1' S = Spacing in ft. = 25 = Soil resistivity in ohm-cm = 24,000 ohm-cm N = No. of anodes = 30

 $R = \frac{.00521(24,000) [\ln 8(7) - 1 + 2(7)]}{7(30)} \ln .656(30)]$




- = 2.8 ohms
- b. Anode Resistance to Backfill: $R = \underbrace{0.00521}_{L} (\ln \underbrace{8L}_{D} - 1)$ L = Length of anode = 5' D = Diameter of anode = 0.25' = Resistivity of Backfill $R = \underbrace{.00521(50)}_{5} (\ln \underbrace{8(5)}_{-1} - 1)$ $\underbrace{.25}_{}$ = 0.212 ohm for 1 anode $R \text{ for 30 anodes} = \underbrace{.212}_{30} = 0.007 \text{ ohm.}$ Total Groundbed resistance=2.8 + 0.007 = 2.807ohms.
- c. Cable resistance

Maximum conductor length for this installation should not exceed 1500 ft.

Use # 1/0 AWG, resistance = .102 ohms/1000 ft. Cable resistance = $1500 \times .102/1000 = 0.153$ ohms Total Groundbed Resistance: 2.807 + 0.153 = 2.96 ohms

d. Rectifier Voltage Rectifier Voltage V_r=<u>IR+2V (Back EMF)</u> .8 reserve factor

Design current output = 30 amperes $V_r = \frac{30(2.96) + 2V}{8} = 113.5$ volts

Use the next larger rating = 120 volts

B. Four Fuel Storage Tanks-Main Exchange Gas Station

- Current requirement test data indicated that a current 0.6 ampere was sufficient for protection at most test points. Protective potentials will be acheived with better current distribution and an additional 50% of direct current, say 1.0 ampere.
- Since the soil resistivity is reasonably high (11000 ohm-cm) and current distribution is very important, a single rectifier and 8 anodes are recommended for installation.
- 3. The weight of anode materials is not a factor due to the small current drain required. Type 3" x 60"



specially treated graphite anodes with calcined petroleum coke backfill are recommended.

4. Groundbed design:

Soil Resistivity = 11000 ohm-cm

 $R = \frac{.00521(11000)}{7} (\ln \frac{8(7)}{1} - 1)$

Resistance of 1 single anode= 24.8+ 0.212 = 25.ohms

Groundbed Resistance = 25/8 anodes = 3.125 ohms say 3.0 ohms.

Rectifier Rating:

1.

Rectifier Voltage $V_r = \frac{IR+2V (Back EMF)}{.8 reserve factor}$

Maximum current drain = 1 ampere $V_r = \frac{(1)(3.125)+2V}{.8} = 6.4$ volts

In order to reduce the stock of spare parts and rectifier maintenance a 10 volt 5 ampere rectifier is recommended for installation.

C. Fuel Storage Tanks at Building 1855

The 4-6000 gallons underground steel tanks near building 1855 have an exposed surface area of 2,060 square feet. Based on a current density of 0.326 ma/sq.ft. as calculated for a similar type tank in the Rifle Range area, these tanks will require:

2060 sq.ft. x .326 ma/sq.ft. = 671.5 miliampers = 0.671 amperes.

 Since the soil resistivity is high (16,000 ohm-cm) an impressed current system is recommended for installation.

 Following the same procedure outlined previously, a 10 Volt-5 ampere rectifier in conjunction with 8 each 3 x 60 treated graphite anodes are recommended for installation.

- D. Fuel Storage Tanks at Bldg. 1775
- The two 16,000 gallons underground steel tanks near building 1785 have an exposed surface area of 1030 square feet. Based on a current density of 0.326 ma. /sq.ft. as calculated for similar type tank in the Rifle Range area, these tanks will require:



1030 sq.ft. x .326 ma = 335.8 ma = 0.336 amperes

- Since the soil resistivity is high (16,000 ohm-cm) an impressed current system is recommended for installation.
- 3. Following the same procedure outlined previously, a 10 volt, 5 amperes rectifier in conjunction with 6 each 3 x 60 treated graphite anodes are recommended for installation.
- II. POL SYSTEM- RIFLE RANGE AREA
- A. Fuel Storage Tank at Gas Station
- 1. Current requirements test data indicated that a current of 0.250 ampere will be required to achieve protective potentials on the 10,000 gallon underground tank in the Rifle Range area.

Tank Dimensions: 8' diameter x 26.5' long Tank Surface area = 767 sq. ft. Current density = $\frac{0.25 \text{ amps}}{767 \text{ sq. ft.}}$ = 0.000326 Amp/sq.ft.

= 0.326 ma/sq.ft.

2. Average Soil Resistivity at 10' depths is 10,000 ohm-cm for economic evaluation purposes, consider 2 alternates:

> Alternate A- Sacrificial sytem Alternate B- Impressed system

Alternate A-

Sacrificial Anodes System

 Weight of anode materials required: Prepackaged magnesium anodes will be used having an estimated deterioration rate of 1-1b per 500 Amp-hr. and an estimated life of 20 years

Weight=20 yrs x $\frac{1-1b}{amp-yr}$ x $\frac{8760hr}{1 yr}$ x 0.25 amp = 87.5 lbs of anode materials

- 2.
- Number of anodes required for 20 years life:

a. Use prepackaged 20 1b elongated magnesium anode.

b. Number = 87.5 lbs x l anode/20lb = 4.37 anodes

 $4.37 \times 1/.75 = 5.83$ anodes; Use 6 anodes.



.75 is the utilization factor meaning when the anode is 75% consumed it will require replacement.

c. Calculated current drain for a 20-D2 Galvomag Galvopack, high potential magnesium anode with a driving potential of 0.9 volt:

$$R = \underbrace{.00521}_{L} (\ln \underbrace{8(L)}_{D} - 1)$$

$$P = \text{Soil Resistivity} = 10,000 \text{ ohm-cm}$$

$$L = \text{Anode Length} = 5'$$

$$D = \text{Anode Diameter} = 0.266'$$

 $R = \frac{.00521 \ (1000)}{5} \ (\ln \frac{8(5)}{0.266} - 1) = 41.8 \text{ ohms}$

I = E/R E = driving potential I = 0.9volt/41.8 ohms = 0.0215 amper/anode

Number = $0.250 \text{ amp } \times \frac{1-\text{anode}}{0.0216 \text{ amp}} = 11.57 \text{ anodes.}$

d. To achieve the desired current drain and a minimum of 20 years life for the system, twelve (12) 20-D2 Galvopack magnesium anodes will be scheduled for installation.

Alternate B. Impressed Current System

1. Weight of anode material required

Specially trated graphite anodes will be used having an estimated deterioration rate of 1-1b per ampere year for an estimated life of 20 years.

Weight = 20 years x $1-1b/amp-yr \times 0.25$ amps = 5 lbs

- 2. Number of anodes required
 - a. The weight of anode materials is not a factor due to the small current drain required; 3" x 60" specially treated graphite anodes with calcined petroleum coke backfill will be utilized.
 - b. For good current distribution and low groundbed resistance, four (4) anodes are recommended for this installation.
- 3. Groundbed design

 $R = \frac{.00521 \mathscr{P}}{L} (\ln \frac{8(L)}{D} - 1)$ $R = \frac{.00521 (10,000)}{7} (\ln \frac{8(7)}{1} - 1)$



Resistance of 1 single anode = 20 ohms.

Groundbed resistance = 20.0 + 0.212 = 20.212/4 anodes = 5.05 ohms

4. Rectifier Rating:

Rectifier Voltage $V_r = \frac{IR+2V (Back EMF)}{.8 reserve factor}$

Allow 1 ampere for current drain $V_r = \frac{(1)(5.05) + 2V}{8} = 8.8$ volts

Use the nearest standard size, 10V-5 amps, air cooled, single phase unit.

- III. POL SYSTEM COURT HOUSE BAY AREA
- A. Fuel Storage Tanks at Gas Station
- 1. Current requirement test data indicated that a current of 0.4 amperes will be required to achieve protective potentials on the 3-6000 gallons underground fuel tanks. Current density required for cathodic protection is 0.4 amp/1545 sq.ft. = 0.000259 ampere = 0.26 ma.
- 2. Since the soil resistivity is high (25000 ohm-cm) and current distribution is important a single rectifier and six (6) anodes are recommended for installation.
- 3. The weight of anode materials is not a factor due to the small current drain required. Type 3" x 60" specially treated graphite anodes with calcined petroleum coke backfill will be utilized.
- Groundbed Design:

Soil Resistivity = 25000 ohm-cm

 $R = \frac{.00521 (25000)}{7} (\ln \frac{8(7)}{1} - 1)$

Resistance of 1 single anode= 56.2 + .212 = 56.4ohms Groundbed Resistance = 56.4/6 anodes = 9.4 ohms Rectifier Rating:

Rectifier Voltage $V_r = \frac{IR+2V (Back EMF)}{.8 reserve factor}$



$$V_r = \frac{(0.4)(9.4) + 2V}{.8} = 7.2$$
 volts

In order to reduce the stock of spare parts and rectifier maintenance a 10 Volt 5 amperes rectifier is recommended for installation

B. Diesel Fuel Storage Tank

 The 30,000 gallon underground diesel tank has a calculated area of approximately 1690 square feet. Based on a current density of 0.326 as calculated for similar type tank, the tank will require:

1690 x 0.326 = 550 milliampers = 0.55 amperes

- Since the soil resistivity is high (25000 ohm-cm) an impressed current system is recommended for installation.
- 3. Following the same procedure outlined previously a 10 volt 5 ampere rectifier in conjunction with 6-3 x 60 specially trated graphite anodes are recommended for installation.
- IV. POL-SYSTEM BEACH AREA
- A. # 2 Fuel Tank at the Steam Plant.
- Current requirement test data indicated that 9.8 amperes were applied to the # 2 fuel tank. Protective potentials were not achieved due to the electrical continuity between the tank and the steam plan. As a result, design calculations are based on previous current requirement tests conducted with consideration for the low soil resistivity.
- Based on a current density of 1.0 ma per square foot and an exposed tank surface area of 767 square feet, the tank will require:

767 sq. ft. x 1.0 ma/sq.ft. = 0.767 amps.

- 3. The low soil resistivity (2500 ohm-cm) is suitable for a sacrificial magnesium anode installation.
- 4. Groundbed design:

a. Design life = 20 years

b. Weight of anode materials:



Weight=20 years x 1-1b/500 amp.yr. x 8760hr/yr x 0.76 amp = 268 lbs.

 $268 \times 1/.75 = 357$ lbs of anode materials, .75 is the utilization factor

c. Minimum number of anodes Assume use of 40-D3 (40 lbs) magnesium anodes:

Number = 357 lbs x 1 anode/40 lbs = 9 anodes.

d. Calculated Current drain for a 40-D3 Galvomag Galvopack high potential magnesium anode with a driving potential of 0.9 V.

 $\frac{R=.00521(2500)}{5} \quad (\ln \frac{8(5)}{.3125} - 1) = 10 \text{ ohms}$

- I = E/R E = driving potential<math>I = 0.9/10 = 0.09 ampere/anode.
- e. To achieve desired current drain and a 20 years life for the system, nine (9) 40-D3 Galvopack magnesium anodes will be scheduled for installation. Combined current ouput of all anodes should be restricted to 0.81 amperes.
- V. POL SYSTEM FRENCH CREEK AREA
- A. # 2 Fuel Tank at Bldg. FC-202
- Current requirement test data indicated that 100 ma. were not adequate to acheive protective potentials on the 10,000 gallon tank. Due to the high soil resistivity in the tank area (66000 ohm-cm) the maximum current drain from the temporary groundbed ' was 100 ma

The exposed surface area of the tank if 767 sq. ft. Based on a current density of 0.326 ma/sq.ft. as calculated at other similar underground tanks; total current requirement will be 0.25 amperes.

- Since the soil resistivity is high a single rectifier and 6 anodes are recommended for installation.
- 3. The weight of anode materials is not a factor due to the small current drain required. Type 3" x 60" specially treated graphite anodes with calcined fluid petrolelum coke backfill will be utilized.
- 4. Groundbed design

Soil resistivity = 66000 ohm-cm



 $R = \frac{.00521(66000)}{7} (\ln \frac{8(7)}{1} - 1)$

Resistance of 1 single anode=148 + 0.212 = 148.2 ohms Groundbed resistance = 148.2/6 anodes = 24.8 ohms.

5. Rectifier Rating =

Rectifier Voltage $V_r = \frac{IR+2V (Back EMF)}{.8 reserve factor}$

 $V_r = \frac{(0.25)(24.8) + 2V}{.8} = 10.3$ volts

Use the next larger rating of 20V-5 amps.

VI. POL SYSTEM- NEW NAVAL HOSPITAL

A. Fuel Storage Tank - New Navy Hospital

1.

2.

3.

Current requirement test data indicated that a current of 0.235 amperes will be required to acheive protective potentials on the 10,000 gallons underground tank.

Tank Dimensions; 10' diameter x 17'-8" long. Tank Surface Area= 712 sq. ft.

Current density = $\frac{.235 \text{ amp.}}{712 \text{ sq.ft.}}$ = 0.00033 amp/sq.ft.

= .33 ma/sq.ft.

Weight of anode materials: Because of the low current requirement and the reasonably low soil resistivity of 4000 ohm-cm near the tank, sacrificial magnesium anodes will be used having an estimated deterioration rate of 1-lb per 500 amp-hr and an estimated life of 20 years.

Weight=20yrs. x 1-1b/500 amp-yr x 8760hr x 0.235 amps = 82.3 lbs of anode materials

- Number of anodes required for 20 year life:
 - a. Use prepackaged 20-D2 high potential magnesium anodes

b. Number=82.31bs x l-anode/20-1b = 4.1 anodes

 $4.1 \times 1/.75 = 5.46$ anodes

.75 is the utilization factor.

Use 6 anodes.



c. Calculated current drain for a 20-D2 Galvopack anodes with a driving potential of 0.9 volt:

 $R = \frac{.00521(4000)}{5} \quad (\ln \frac{8(5)}{5} - 1) = 16.7 \text{ ohms.}$

 $I = \frac{E}{R}$ E = Driving potentialI = 0.9 volt/16.7ohm = 0.054 amp/anode

Number of anodes required for 0.235 amperes:

0.235 amp x $\frac{1-\text{anode}}{.054}$ = 4.35 anodes

- d. To achieve the desired current drain and a minimum of 20 years life for the system, six (6) 20-D2 Galvopack magnesium anodes will be scheduled for installation. Combined current output of all anodes should be restricted to a maximum of 0.350 amperes.
- B. Heating Oil Storage Tanks New Naval Hospital
- 1. The 5 heating oil underground steel tanks at the New Naval Hospital have an exposed surface area of 5030 square feet. Based on a current density of 0.00033 ampere per square foot as calculated for the 10,000 gallon MOGAS tank in the same area, these tanks will require:

 $5030 \times .00033 = 1.66$ amperes.

- 2. The low soil resistivity (2600 ohm-cm) is suitable for a sacrificial magnesium anode installation.
- 3. Groundbed design
 - a. Design life = 20 years.
 - b. Weight of anode materials:

Weight=20yrs x 1.66 amp x 1-1b/500amp-yr x 8760 hr/yr= 580 lbs of anode material

 $580 \times 1/.75 = 770$ -lbs of anode material 0.75 is the utilization factor.

c. Minimum number of anodes required:

Assume use of 40-D3 (40 pounds) magnesium anodes:

Number = 770 lbs x l-anode/40 lb = 19.25 say 20 anodes.



d. Calculated current drain of a 40-D3 Galvomag, Galvopack high potential magnesium anode with a driving potential of 0.9 volt.

Average soil resistivity at 10' depth = 2600 ohm-cm.

- $R = \frac{.00521(2600)}{5} \quad (\ln \frac{8(5)}{.3125} 1)$
 - = 10.4 ohms
- $I = \frac{E}{E}$ E = driving potential

I = 0.9/10.4 = 0.086 amperes/anode.

e. To achieve desired current drain and a 20 years life for the system, twenty four (24) 32-D3 Galvopack magnesium anodes will be scheduled for installation. Combined current output of all anodes should be restricted to a maximum of 2.30 amperes.

VII. WATER DISTRIBUTION SYSTEM

 Based on a current density of 0.0015 ampere per square foot, current requirement for different standard pipe joints will be as follows:

Dimension	Current requirement
4" x 20'	0.032 A
6" x 20'	0.047 A
8" x 20'	0.063 A
10" x 20'	0.078 A
12" x 20'	0.094 A
14" x 20'	0.109 A
20" x 20'	0.157 A



- Because of soil resistivity variations and the lack of electrical continuity, anodes are sized for each individual joint.
- Weight of anode materials required for a 6" x 20' joint.
- a. Anode life = 20 years weight = 20 yrs x $\frac{8760 \text{ hr}}{\text{yr}}$ x $\frac{11b \text{ x}}{500 \text{ amp-hr}}$ x $\frac{1}{.85}$ = 19.371bs
- b. Select (1) 20-D2 Valvopack magnesium anode for installation on each 6" x 20' joint
- c. Anode Resistance:

 $R_{=} \frac{.00521()}{L} (\ln \frac{8}{D} - 1)$

 $= \frac{.00521()}{5} (\ln \frac{8(5)}{.266} - 1) = 0.004$

Maximum current drain depends on soil resistivity.

 $\frac{I}{R} = \frac{\text{Driving Potential}}{R} = \frac{0.09V}{.004}$

For 1000 ohm-cmI = .225 amperes

Therefore (1) 20-D2 anode can be used on 1 joint of 6" x 20'pipe in soil resistivities up to 5000 ohm/cm.

 Following the above procedure the following tables were prepared:

4" x 20'

Maximum Soil Resistivity ohm-cm	No. of magnesium Anodes Re.	Maximum Current Ouput "Amperes"
1000	1-20D2	0.215
2000	1-20D2	0.1076
3000	1-20D2	0.072
4000	1-20D2	0.054
5000	1-20D2	0.043





	<u>6" x 20'</u>	
1000 2000 3000 4000 5000	1-20D2 1-20D2 1-20D2 1-20D2 1-20D2 1-20D2	0.215 0.1076 0.072 0.054 0.043
	<u>8" x 20'</u>	
1000 2000 3000 4000 5000	1-32-D3 1-32-D3 2-20D2 2-20D2 2-20D2 2-20D2	0.192 0.096 0.144 0.108 0.086
	<u>10" x 20'</u>	
1000 2000 3000 4000 5000	1-40D3 1-40D3 1-40D3 2-20D2 2-20D2	0.2432 0.122 0.081 0.108 0.086
1	<u>12" x 20'</u>	
1000 2000 3000 4000 5000	1-48D5 2-20D2 2-20D2 2-20D2 2-20D2 2-20D2	0.152 0.215 0.143 0.1076 0.086
	<u>14" x 20'</u>	
1000 2000 3000 4000 5000	1-48D5 1-40D3 2-20D2 2-20D2 2-20D2 2-20D2	0.152 0.121 0.224 0.168 0.135
	20" x 20'	
1000 2000 3000 4000 5000	2-40D3 2-40D3 2-40D3 2-40D3 2-40D3	0.484 0.242 0.161 0.112 0.090



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APPENDIX E

COST ESTIMATES



NAVFAC 11013/7 (1-78) Supersedes NAVDOCKS 2417 and 2417A	COST	ESTIM	ATE		DAT	E PREPARED	laurer	1	
MCB CAMP LEJEUNE N.C.				CONSTRUCTION CONTRACT NO.					
CATHODIC PROTECTION SURVEY			ESTIMATED BY MENENDEZ-DONNELL & ASSOC. STATUS OF DESIGN PED 100 DESIGN				CATEGORY CODE NUMBER		
ITEM DESCRIPTION	QUANT	QUANTITY		MATERIAL COST		LABOR COST		ENGINEERING ESTIMATE	
FUEL FARM-INDUSTRIAL AREA	NOMBER	UNIT	UNIT COST	TOTAL	UNIT COST	TOTAL	UNIT COST	TOTAL	
1 - 120V 40A OIL IMMERSED RECT	. 1	EA	3050	3050	950	950		4000	
2 - 3 × 60" TREATED GRAPHITE ANODE	5								
W/5-#8HMWPE LEADWIRE	30	EA	76	2280	180	5400	C. E.	7680	
3 - CALCINED FLUID PETROLEUM COKE	= 11,500	LB	0.32	3680		1200		4880	
4 - * 10 HMWPE HEADER CABLE	1,500	FT	0.95	1425	1.25	1875		3300	
5- EPOXY RESIN SPLICE KITS &			••• .a.	the material of the				5900	
PRESSURE CONNECTION	30	EA	14	420	22	660		1080	
6 - A/C POWER CONNECTION		LOT	250	250	550	550	tennen er	300	
1- TEST STATION	14	EA	65	910	180	2520	and a second	3430	
8- MISCELLANEOUS	1	LOT	300	300	800	800	REAL,	1100	
9-FIELD ENGINEERING & SUPERV								2440	
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PROJECT TITLE			MENENDEZ - DONNELL & ASSOC.					CATEGORY CODE NUMBER	
CATHODIC PROTECTION SURVEY	1 34 6 4		PED 30% 100% FINAL Other (Specify) 5				JOB ORDER NUMBER		
ITEM DESCRIPTION	QUAN NUMBER	TITY	MATERIAL COST		LABOR COST		ENGINEERING ESTIMATE		
RIFLE RANGE - FUEL TANK @					ONTICOST	IOTAL	JNIT COST	TOTAL	
GAS STATION, SACRIFICIAL SYSTEM								the second second	
1- 20-02 PREPACKAGED MAGNEGIUM									
ANODES W/ 10- #12 TW LEAD WIRE	12	EA	66	792	165	1980		2772	
2- \$ 10 ANG HEADER CABLE	150	FT	0.15	23	1.25	188		- 211	
3- CRIMP CONNECTIONS & SPLICE	12	EA	0.50	. 6	12	144		150	
4- TEST STATION	1	EA	65	65	180	180		215	
5- MISCELLANEOUS	1	LOT	150	150	400	400		550	
6- ENGINEERING & SUPERVISION								1025	
7 - OFFICE ENGINEERING & SUPERVISION								1025	
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MCB, CAMP LEJEUNE, N.C.			ESTIMATED BY					
PROJECT TITLE		••• ••• ••	MENEN	DEZ-DON	INELL &	ASSOC.	CATEGOR	T CODE NUMBER
CATHODIC PROTECTION SURVEY			PED PED	IGN 30% 100%	FINAL XO	iner (Specify) STUDY	JOB ORDE	A NUMBER
ITEM DESCRIPTION			MATERIAL COST		LABOR COST		ENGINEERING ESTIMATE	
RIFLE RANGE - FUEL TANK		0	-	TOTAL	UNIT COST	TOTAL	UNIT COST	TOTAL
IMPRESSED CURRENT SYSTEM								
1- IOV- SA AIR COOLED RECTIFIER	1.	EA	510	510	575	575		1085
2- 3"x CO" TREATED GRAPHITE ANODES								1000
W/101-*8 HMWPE LEAD WIRE	4	EA	76	304	180	720		1074
3- CALCINED FLUID PETROLEUM COKE	1,600	LB	0.32	512		160		1.72
4-#4 HMWPE HEADER CABLE	200	FT	0.49	98	1.25	250		210
5- EPOXY RESIN SPLICE KIT &				- to metalise - Sign	1.00%	670		540
CRIMP CONNECTION	4	EA	14	56	22	88		144
6 - POWER CONNECTION	1	EA	200	200	85	85		025
7-TEST STATION	1	EA	65	65	180	180		200
8- MISCELLANEOUS	1	LOT	200	200	300	300		500
9 - FIELD ENGINEERING & SUPERV.								1025
0- OFFICE ENGINEERING & REPORT								1000
11 - DRAFTING & SECRETARIAL			1.000			· · · ·		600
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CATHODIC PROTECTION SURVEY		PED 30% FINAL OTHER (Suecily) STUDY JOB OR					ER NUMBER		
ITEM DESCRIPTION		QUANTITY NUMBER UNIT		MATERIAL COST		OR COST	ENGINEERING ESTIMATE		
NEW NAVAL HOSPITAL & BEACH AREA	14 E				UNITCOST	TOTAL	UNIT COST	TOTAL	
1- 20-02 PREPACKAGED MAGNEGIUM									
ANODES W/10! #12 TW LEAD WIRES	6	EA	66	396	165	990		1386	
2- 40. D3 PREPACKAGED MAGNESIUM					-		· ••	-	
ANODES W/10' - #12 TW LEAD WIRES	29	EA	126	3654	180	5220		8 871	
3- #10 TH HEADER CABLE	800	FT	0.15	120	1.25	1000	• • •	1120	
4- TEGT GTATION	5	EA	65	325	180	900		1225	
5- CRIMP CONNECTIONS & SPLICE	35	EA	1.00	35	12	420		455	
G- MISCELLANEOUS	1	LOT	600	600	1100	1100		1700	
7- FIELD ENGINEERING & SUPERVISION								1 850	
8- OFFICE ENGINEERING & REPORT								1 800	
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				IGN 30% 100%	FINAL X OI	HE (Specify) STUDY	JOB ORDI	JOB ORDER NUMBER
ITEM DESCRIPTION		TY	MATE	RIAL COST	LAB	OR COST	ENGINEER	RING ESTIMATE
FUEL STORAGE TANKS	NOMBER	UNIT	UNIT COST	TOTAL	UNIT COST	TOTAL	UNIT COST	TOTAL
1- IOY 5A AIR COOLED RECTIFIER	4	EA	510	2040	575	2300		4340
2- 201-5A AIR GODLED RECTIFIER		EA	510	510	575	575		1085
3- 3"x 60" TREATED GRAPHITE ANODES								1,000
W/101-*3 HMWPE LEAD WIRE	32	EA	76	2432	180	5760		8.192
4. CALCINED FLUID PETROLEUM COKE	10,500	LB	0.32	3360		1400		4.760
5- #4 HMWPE HEADER CABLE	1,200	FT	0.49	588	1.25	1500		2088
6 - EPOXY REGIN KITS & SPLICE CONNECTION	32	EA	14	448	22	104		1.192
7- POWER CONNECTION	5	EA	200	1000	85	425		1425
B- TEST STATION		EA	65	325	180	900		1.225
9 - MISCELLANEOUS		LOT	150	150	250	1250		2.000
10 - FIELD ENGINEERING & SUPERV.			A. Meesta					5.000
11 - OFFICE ENGINEERING & REPORT		N. K.	1.2					3.500
12- DRAFTING & SECRETARIAL								1900
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ACTIVITY AND LOCATION			CONSTRUCTION	CONTRACT NO.			IDENTIFIC	ATION NUMBER	
MCB, CAMP LEJEUNE, N.C.			ESTIMATED BY						
PROJECT TITLE			MENENDEZ · DONNELL & ASSOC.					CATEGORY CODE NOMBER	
CATHODIC PROTECTION SURV	EY		PED	30% 100%	FINAL OU	er (Specify) STUDY	JOB ORDE	RNUMBER	
ITEM DESCRIPTION QUANTITY			MATE	RIAL COST	LAB	OR COST	ENGINEERING ESTIMATE		
MAIN EXCHANGE FUEL TANKS			0001 0001		UNITCOST		NIT COST	TOTAL	
1-10V 5A, AIR COOLED RECTIFIER	1	EA.	510	510	575	575		1.085	
2-3"XGO" TREATED GRAPHITE ANODES		EA.	16	609	180	1440		nnha	
WIO- #8 HMWPE LEAP WIRE					1.00			2,040	
3. CALCINED FLUID PETROLEUM	2,600	Ľb.	0.32	832		400	6.11	1.720	
4- #4 HMWPE HEADER CABLE	300	FT.	0.49	147	1.25	375		520	
5- EPOXY RESIN KITS ESPLICE CONNECTION	8	EA.	14	112	22	176		788	
5- POWER CONNECTION	1	EA.	200	200	85	85		285	
T- TEST STATION	4	EA.	65	260	180	720	ann an an Eil ann	980	
8- MISCELLANEOUS		LOT	150	150	250	250		400	
9-FIELD ENGINEERING & SUPERVISION								1200	
10- OFFICE ENGINEERING & REPORT								1.000	
11- PRAFTING & SECRETARIAL							* ***	600	
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TOTALS				2,819		4.021		9.640	

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APPENDIX F

CORROSION AND CATHODIC PROTECTION



CORROSION AND CATHODIC PROTECTION THEORY

Corrosion is an electro-chemical process or transformation of energy resulting in the metal of a structure in contact with an electrolyte going into solution, or reverting to its natural status as an oxide form. There is a great deal of stored energy in a piece of metal and it is not at all in accordance with the laws of nature for that piece of metal to remain intact--in fact, it cannot exist without some type or degree of maintenance by man.

There are, generally speaking, two main forms of corrosion--electrolytic and galvanic. Electrolysis is usually construed to mean the process of a stray electrical current being impressed upon a buried structure from an external and metallically unconnected source such as an electric railway (Figure 1). The current, usually relatively great in magnitude, supposedly confined to the rail as a return encounters high resistant joints, takes the path of least resistance to nearby piping, follows the pipe line back to the proximity of the source, at which point the current is discharged from the line carrying iron particles into solution with it. Due to the quantity of current usually involved, this type of corrosion is usually manifested in severe metal loss in the area of current discharge. Any uncontrolled current from a D.C. current source can result in detrimental interference effects on foreign structures within the area of influence of the D.C. source.

Galvanic corrosion is the result of the formation of galvanic cells upon the structure itself and independent of external power sources. Basic forms of galvanic cells exist as: (a) dissimilar connected metals in a common electrolyte, (b) a continuous metal structure exposed to dissimilar electrolytes, and (c) a combination of the above conditions. It is this form of corrosion which plays the major role in deterioration of underground structures in most areas.

The galvanic cell involving dissimilar metals can perhaps best be illustrated by referring to these examples taken from the Electromotive Force Series of Metals Table (Figure 2). This table is a comparative index of the solution potential or activity level of various metals ranging from potassium which has the highest relative potential to the noble metals of silver and gold which are very stable and thus reflect the lowest solution potentials. For practical purposes, the most common metals for underground construction and cathodic protection are shown. Magnesium, with a potential of -2.34, is anodic to zinc, with a



potential of -0.762. Zinc, in turn is anodic to iron, with a potential of -0.044. Iron, with a potential of -0.044, is anodic to copper, with a potential of +0.345. The term anodic is of Greek derivation meaning "up way" and indicates that the metal which has the higher potential will give up current (thus dissipating itself) to the lower potential metal which is termed cathodic or the cathode.

The common flashlight battery is a galvanic cell composed of a zinc outer case, an electrolyte, a carbon rod, and an external circuit (Fig. 3). In this case, the zinc has the higher potential and acts as the anode with the carbon rod being the cathode. When the external circuit is closed through the metallic case of a flashlight, current flows from the zinc outer case, through the electrolyte to the carbon rod, and thence through the light bulb filament. As the metallic ions go into solution, water in the electrolyte is disassociated, the zinc combining with the hydroxyl ion to form an oxide, and the atomic hydrogen released to migrate to the cathode.

Common examples of this type of galvanic cell encountered in everyday construction of underground structures are a brass fitting between steel section (Fig. 4), steel connected to cast iron, steel pipe in contact with cinders (Fig. 5), bright metal from wrench or tong from scratches (Fig. 6), mill scale patches on pipe (Fig. 7), and new pipe installed as replacement between old sections of pipe.

The other basic galvanic cell is one consisting of a common metal in dissimilar electrolytes (Fig. 8). In this case, the electrolyte surrounding the metal determines which portion of the metal is anodic and which is cathodic. The current flow is from the metal in contact with the lower resistivity electrolyte to the portion of metal in a higher resistivity environment. This case is, of course, similar to our underground pipe lines composed of the same metal, but traversing a heterogeneous mixture of soils such as sand, sandy loam, clay, loam, rock, gypsum beds, salt beds, etc. The oxygen content and moisture conditions will also vary radically for different soil types encountered. Each change of soil characteristic such as the frequency, and the degree of change of resistivity, has a great role in determining the severity and extent of corrosion.

Examples of these conditions are dramatized in Figure 9, which illustrates a continuous metal pipe in contact with a moisture retentative (thus relatively low resistivity), clay electrolyte, and also a well-drained (thus higher resistivity) sandy loam electrolyte. Current discharge is



initiated in the lower resistivity soil area with the adjacent pipe surfaces receiving the current, and the pipe wall serving as the external circuit back to the source of the galvanic cell at the corroding area. Figures 10 and 11 illustrate the dissimilarity of soil conditions which can result from normal excavation and backfill procedures of buried structures; also, the dissimilarity of electrolyte conditions encountered due to oxygen availability and presence as a result of normal construction practices.

A typical example of numerical soil resistivity value relationships over an extent of pipe line right-of-way is shown in Figure 12. Although a large percentage of detrimetnal corrosion is normally associated with the low soil resistivity ranges, severe corrosion does occur in the medium and high range categories. Thus, the frequency and magnitude of electrolyte change must be considered rather than relying solely on categorized numerical ranges.

Corrosion results are apparent in several forms -- the most common being scaling, pitting, patching, graphitization, and oxide films. Some less common forms are failure within the crystaline structure itself and stress corrosion. Uniform scaling, or exfoliation, is usually associated with some of the older laminated types of pipe construction. The severity of metal loss depends essentially on the ratio of anodic area to cathodic area. In other words, if there is a small anodic area between two large cathodic areas, the small anodic area will be discharging current in quantities large enough to protect the two large cathodic Since the area of current discharge is small, it areas. follows that the metal will be removed in this area at an accelerated rate. However, if the anodic area was relatively large in comparison with the cathodic area, the penetration process would proceed much slower as it would be taking place over a much larger area. When it is realized that one ampere of D.C. current flowing continuously for a period of one year can drive 20 pounds of steel into solution, it can be ascertained that very small quantities of uncontrolled current discharge can cause failure of a thin wall metallic structure within a relatively short time.

Corrosion prevention is normally accomplished by the following procedures:

 Judicious choice of construction materials and procedures with respect to corrosion mitigation for new construction.



2. Protective coatings.

3. Cathodic protection.

On new construction, many corrosion problems of the future can be prevented during the design stage of proposed faciliites. The type of metal most suitable for handling a given product, the type of surface treatment for the metallic structure, provisions for electrical isolation of new systems from old or foreign systems, and minimizing or avoiding coupling of dissimilar metals are but a few of the decisions which merit consideration during the project planning phase.

Protective coatings are recognized as a basic weapon in the battle against underground corrosion. It is known that if the metal of a structure does not contact an electrolyte, no corrosion will take place. Thus, the use of coatings is widespread, the desire being a coating material which is an impervious, inert substance, unaffected by temperature variance, mechanically sturdy enough to withstand soil and cyclic stress to which it is subjected underground, as well as potential damage from handling during transportation and Commonly used coating materials consist of construction. asphalt and coal tar enamels, asphalt and coal tar mastics, polyethylene and polyvinyl chloride tape applications, micro-crytaline wax compounds, and extruded plastic jackets or sleeves. Coating efficiences of the pipe line coatings in place are dependent not only on the material used, but also the care with which it was applied and the care exercised during structure installation. It is virtually a physical impossibility for any coated structure in place and backfilled to be without minute faults or "holidays", with small bare metal surfaces thus exposed and in direct contact with the surrounding soil or electrolyte. This situation is a classic example of the condition previously discussed concerning ratios of anodic and cathodic areas. Since the exposed metallic area at any coating fault will be 'relatively small compared to coated or cathodic areas surrounding it, corrosion activity will be concentrated on the small bare metallic area and early metal loss and penetration may be reasonably anticipated unless further protective steps are taken. In addition, all coating materials are subject to deterioration with time, thus exposing more metal surface to the corrosion process.

The accepted supplement to coating procedures is that of applying cathodic protection to the coated structure. In general, cathodic protection is a process whereby adequate quantities of D.C. current are impressed upon a given



structure to overcome the quantities of galvanic current generated and being discharged from the structure. This procedure is accomplished through the use of external current sources; either, galvanic anodes or impresssed current systems. Galvanic anodes normally consist of zinc or magnesium alloys of varying shapes and weights to accommodate differing soil resistivity values, current outputs, and design life. In both cases, the anode metal is more active or higher in the electromotive series than the steel structure to which it is attached. Thus, (Fig. 13) a large galvanic cell has been deliberately created with the metal from the sacrificial galvanic anode being dissipated to prolong the life of the structure to which it The current flow, electrically speaking, is is attached. from the sacrificial anode through the earth onto the structure and is returned to the source through the leadwire connected to the structure and the anode.

The same principle holds true for impressed current systems (Fig. 14), except that in this case power is being derived from some external source such as rectifier units which convert A.C. electrical power to D.C. current, or possibly thermoeletric units which convert heat to electric power. The D.C. current is then routed through a groundbed composed of graphite rods, cast iron rods, or junk steel, and thence through the earth to the structure to be protected. Once again, a low resistant return path is provided between the structure and the power source to complete the circuit and to provide controlled current drainage from the structure.

Cathodic protection in various forms and to varying degrees can be applied to old existing structures as well as new construction.

Naturally, the cost of providing complete overall protection to bare structures involves a much greater expenditure than for similar coated structures due to the greater exposed surface area involved on the bare structures. Thus, partial or spot protection at areas subject to deterioration, as indicated by past history or investigative procedures, is often the course followed to reduce maintenance cost and commodity loss, and to prolong useful life of the structure or system.

In any case, whether on new construction or existing facilities, the use of cathodic protection must be justified economically. Since both the initial investment and projected operating costs of cathodic protection are directly dependent upon the design and effectiveness of the installation, it is of great importance that the type of



protective system utilized, amount of current required, and location of the protective current systems must be determined by thorough preliminary field investigation conducted by experienced personnel. Many survey techniques, interpretation standards, and an array of specialized instrumentation are utilized in determining the most economical and practical protective design for providing cathodic protection to a given system or structure. Upon completion of any protective installation, the system must be adjusted and a thorough checkout conducted to determine that adequate protection is being realized over the entirety of the pertinent structure; further, that any detrimental interference effects on foreign or isolated structures are detected and removed.

In as much as electrical grounding systems frequently complicate cathodic protection efforts and contribute to corrosion of other underground structures, possible improvement of grounding procedures and effect of stray current on underground electrical structures merit the following brief discussion.

In general, electrical grounding systems must be comprised of materials that are good electrical conductors with sufficient area in contact with the soil to provide resistance of the current path within the allowable limits, and to be resistant to the corrosion process. The major material utilized for grounding systems in the past has been copper due to its excellent conductance characteristics, reasonable cost, and corrosion resistant properties. As long as overhead power transimission lines utilizing wooden supports were used, very little corrosion damage was apparent from this procedure. However, with the advent of lead sheath cable, armored cable, and galvanized conduit for underground installation, this situation has changed considerably. Potential differences, due to galvanic couples of some of the most commonly used metals for underground electrical construction, are presented in Figure 15. As indicated, the commonly used metals are all anodic to copper, i.e., when coupled with copper in a common electrolyte, the metals will be dissipated to provide current to the copper to which they are attached. Probably the most serious situation here is the couple between lead and copper where even though the potential difference is not as great as indicated for the other couples, the dissipation rate of lead, approximately 75 pounds per ampere year of current, becomes an important' factor.

Conditions being what they are today, considerable thought for grounding procedures should be given to utilization of



other metals for grounding materials, the two most common substitutes being zinc and high silicon cast iron anodes. Zinc anodes are generally considered more attractive because they not only provide a degree of protection to metals to which they are attached due to being higher on the electromotive series of metals, but also they exhibit relatively long effective life in most environments. Of interest is a comparison of grounding rod resistance values between standard copper and zinc grounding rods in varying soil resistivity ranges. This comparison, as presented in Figure 16, indicates the effectivness of the zinc anode, particularly when surrounded by a prepared backfill material. Number, spacing, and configuration of grounding rods to provide a specified resistance can be readily determined in most cases when the resistivity of an electrolyte has been acquired through measurements, based upon design data for zinc anodes. High silicon content cast iron anodes are less attractive due to the galvanic couple between the cast iron alloy and steel. Although the potential difference between the two is not great, being in the neighborhood of 0.10 volt, the steel pipe is nevertheless anodic to the cast iron anode.

Another important aspect of choice of grounding system materials involves the application of cathodic protection to underground facilities within the area. In case of a copper grounding system in contact with piping or conduit to be cathodically protected, it is not uncommon to encounter current requirements 40 to 50 times as great to provide protection for both the copper grounding system and the piping as would be required to protect the piping alone if the copper grounding system was not connected to it. On the other hand, zinc grounding system under the same circumstances would actually supplement the cathodic protection system. In many areas, involving both plant piping and grounding systems, the proper choice of grounding materials thus becomes a decision of major economical importance.

Often a piping system also serves as part of a grounding system. Once again, the coupling of a copper grounding system with steel piping results in dissipation of the steel and should be avoided. In addition, today's standard acceptance of high resistance coatings for pipe line construction actually provides, in many cases, a very poor grounding device.

Neutral conductors for underground electrical distribution systems often consist of bare copper cables with the neutrals of transformers and electrical apparatus housings frequently grounded to the neutral conductor. Water piping



for water-cooled transformers and lead-sheath cables is also often grounded to the neutral conductor cable. Once again, the galvanic couples and resulting potential differences between copper and steel and copper and lead is encountered and deterioration of both the steel water piping and lead sheath cable may be reasonably anticipated. The answer to this problem appears to be a neutral conductor provided with a polyethylene or polyvinyl direct burial jacket which will provide insulation between the copper conductor and the earth, and also provide additional self-contained grounding rods.

Any underground power cable equipped with an adequate polyvinyl or plyethylene jacket will not be influenced by stray current from cathodic protection systems or other stray current sources. Certainly, the lead sheath cable, which parallels a cathodically-protected structure or lays within the area of influence of cathodic protection installations, is receptive to pickup and uncontrolled discharge of stray current resulting in metal deterioration. Interference testing and adequate bonding procedures are the answers to this problem. Lead sheath cable installed in metallic or non-metallic duct systems is not subject to stray current influence, but may be subject to galvanic corrosion action at points within the ducts at which moisture may collect.

Any metallic objects such as pole anchors, grounding rods, cables, or grids which fall within the area of influence of a D.C. current source are exposed to varying degrees of deterioration depending largely upon the metals involved, size of structure, and their proximity to the D.C. current source. In cathodic protection installations, judicious placement of current sources, consistent with design requirements of the structure or system to be protected, is taken into consideration to minimize the possibility of interference on foreign structures. Prior to adjustment and checkout of a protective system, native state potential values on all foreign structures within the area of influence of the current source should be acquired. Upon energizing and adjusting the protective system, potential measurements on the foreign structures involved are again acquired to determine any effects being experience from stray current. In the event that detrimental interference effects on a foreign structure are detected, the situation is relieved by either providing a controlled resistance bond from the affected structure to the current source or providing the affected structure with a small protetive system of its own, normally in the form of self-contained sacrificial anodes. The problems involved, particularly in congested areas involving a number of utilities with the



effects of stray current or interference can be complex in nature and costly in results, unless corrected. As in the case of design, installation, and checkout of protective systems, the detection and correction of interference problems can best be solved by personnel experienced in the specialized field of corrosion mitigation.

1





ELECTROLYSIS CORROSION

FIGURE 1



ELECTROMOTIVE FORCE SERIES

		Standard Electrode Potential
Electrode	Reaction	E ^O (Volts), 25 ^O C
Nagnesium -	Ng ⁺⁺ +2e ⁻	-2.34
Zinc	Zn ⁺⁺ +2e ⁻	-0.762
Iron	70 ⁺⁺ +20 ⁻	-0.440
Lead	Pb++ +22-	-0.126
Ilydrogen	ZH ⁺ +20	-0.00
Copper	cu ⁺ _e ⁻	+0.522



2

A-11





1-12





CORROSION CAUSED BY DISSIMILAR METALS

FIGURE 4








A-15

BRIGHT METAL BY PIPE WRENCH

CORROSION CAUSED BY DISSIMILARITY OF SUR-FACE CONDITIONS

FIGURE 6





PITTING DUE TO MILL SCALE







11/11/2 CATHODE AREA CATHODE AREA. LOAM-SANDY AM SAME PIPE WALL 1. CORROSION CAUSED BY DISSIMILAR SOILS FIGURE 9





CORROSION CAUSED BY MIXTURE OF DIFFERENT SOILS

A-19





A-20

CORROSION CAUSED BY DIFFERENTIAL AERATION OF SOIL









A-22





SCHEMATIC DIAGRAM OF CATHODIC PROTECTION OF BURIED METALS

FIGUES 14



Galvanic Couple	Voltage Difference Volt
Iron-copper .	0.55
Aluminum-coppor	· 155
Lead-coppar	0.45
Zinc (galvanizing)-copper	0.99
	····

GALVANIC COUPLE POTENTIALS





RESISTANCE OF ZINC ANODE VS COPPER CLAD GROUND RODS



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APPENDIX G

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PHOTOGRAPHS



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APPENDIX H

DRAWINGS





TEST PROCEDURE

1

- I. ESTABLISH POSITIVE ELECTRICAL CONTACT TO THE PIPE AT EACH EXTREMITY OF SECTION TO BE TESTED.
- 2. WITH THE SWITCH AT (A) OPEN AND CLOSED, ELECTRICAL CONTINUITY FROM TEST STATION IS INDICATED WHEN EI AND E2 ARE THE SAME MAGNITUDES.
- 3. WITH THE SWITCH AT (A) OPEN AND CLOSED, ELECTRICAL DISCONTINUITY FROM TEST STATION TO TEST STATION IS INDICATED WHEN EI AND E2 ARE DIFFERENT MAGNITUDES.

	MDA MENENDEZ - DONNELL a Associates, INC. GCPS GENERAL CATHODIC GCPS PROTECTION SERVICES, INC.					
		ELECTRICAL CONTINUITY TEST UNDERGROUND PIPELINE				
		DES DR H.D.V.	CK	DWG NO. REV		




	NOTE: For legend and symbols see drawing NO. 6148-5000	
A A A		· .
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O C E V K	A D A MENENDEZ · DONNELL ATLANTIC DIVISION	
TLANTIC	IVI DA & ASSOCIATES, INC. GCPS GENERAL CATHODIC GCPS PROTECTION SERVICES, INC. CATHODIC PROTECTION SURVEY	
	MARINE CORPS BASE, CAMP LEJEUNE, NORTH CAROLINA SOIL RESISTIVITY SURVEY & PIPE/SOIL POTENTIAL SURVEY FOR WATER SYSTEM (AREA 4)	
MAP	DES CK. J. MESZAROS DWG. NO. REV. DR. J. CRUZ APP. SCALE GRAPHIC DATE JAN. 14, 1985 6148-5003	

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LEGEN	ID	
0	SOIL RESISTIVITY TEST POINT	
Ō	PIPE/SOIL POTENTIAL MEASUREMENT	
	SOIL SAMPLE LOCATION	
Δ	WATER SAMPLE LOCATION	
R-	EXISTING RECTIFIER	
GB.	EXISTING GROUNDBED	
(R·	PROPOSED RECTIFIER	
GB.	PROPOSED GROUNDBED	
\bigcirc	EXISTING ELEVATED WATER TANK	
	EXISTING WATER LINE	· .
┝╺┝	EXISTING FIRE HYDRANT	
- - >	EXISTING GATE VALVE	
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AREA REFERENCE

AREA NO.	NAME
2	INDUSTRIAL AREA
3	HADNOT POINT 2
4	HADNOT POINT I
5	OLD NAVAL HOSPITAL
6	OFFICER'S QUARTERS
7	PARADISE POINT
8	BERKELEY MANOR
8 ·A	WATKINS VILLAGE
9	MIDWAY PARK
	TARAWA TERRACE I
	TARAWA TERRACE II
(12)	TRAILER PARK
13	CAMP KNOX
(14)	MONTFORD POINT
(15)	CAMP GEIGER
(16)	GEIGER TRAILER PARK
(17)	RIFLE RANGE
18	COURTHOUSE BAY
(19)	ONSLOW BEACH
20	FRENCH CREEK
(21)	NEW NAVAL HOSPITAL

		GR	APHIC SC	ALE	•	
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DAME	NENDEZ	DONNELL		ATLAN		IVISION

MDA MENENDEZ · DONNELL GCPS GENERAL CATHODIC GCPS PROTECTION SERVICES, INC	ATLANTIC DIVISION NAVAL FACILITIES ENGINEERING COMMAND NORFOLK, VIRGINIA				
CATHODIC PRO	TECTION SURVEY				
MARINE CORPS BASE, CAMP LEJEU	NE, NORTH CAROLINA				
STATION MAP					
DEB. CK. J. MESZAF DR. J. CRUZ APP. SCALE GRAPHIC DATE JAN. 14	DWG. ND. REV. 1985 6148-5000				



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FUEL TANK AT GAS STATION



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MAIN EXCHANGE GAS STORAGE TANK

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425	
VENT LINE PUMP	
(423) V 10,000 GALS	
GAS TANK AT RIFLE RANGE AREA	
NOT TO SCALE	
UNDERGROUND TANK # 2	
(453) (454) (451)	
N AFD	
Building FC-202	
FUEL TANK AT FRENCH CREEK AREA	
	•
<u>NOTE:</u>	
FOR LEGEND AND SYMBOLS SEE DRAWING NO. 6148-5000	
MDA MENENDEZ · DONNELL BASSOCIATES, INC. GCPS GENERAL CATHODIC BROTECTION SERVICES, INC. ATLANTIC DIVISION NAVAL FACILITIES ENGINEERING COMMAND	
CATHODIC PROTECTION SURVEY	
MARINE CORPS BASE, CAMP LEJEUNE, NORTH CAROLINA SOIL RESISTIVITY SURVEY & PIPE/SOIL POTENTIAL SURVEY FOR UNDERGROUND FUEL TANKS	
DES CK. J. MESZAROS DWG. NO. REV. DR. J. CRUZ SCALE NOT TO SCALE DATE JAN. 14, 1985 6148-5020	

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